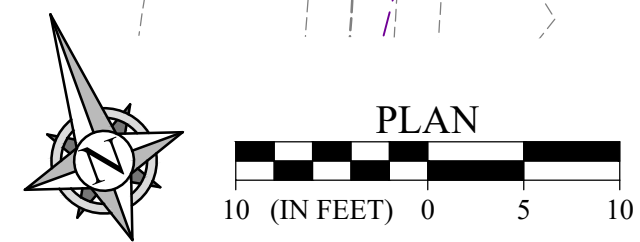
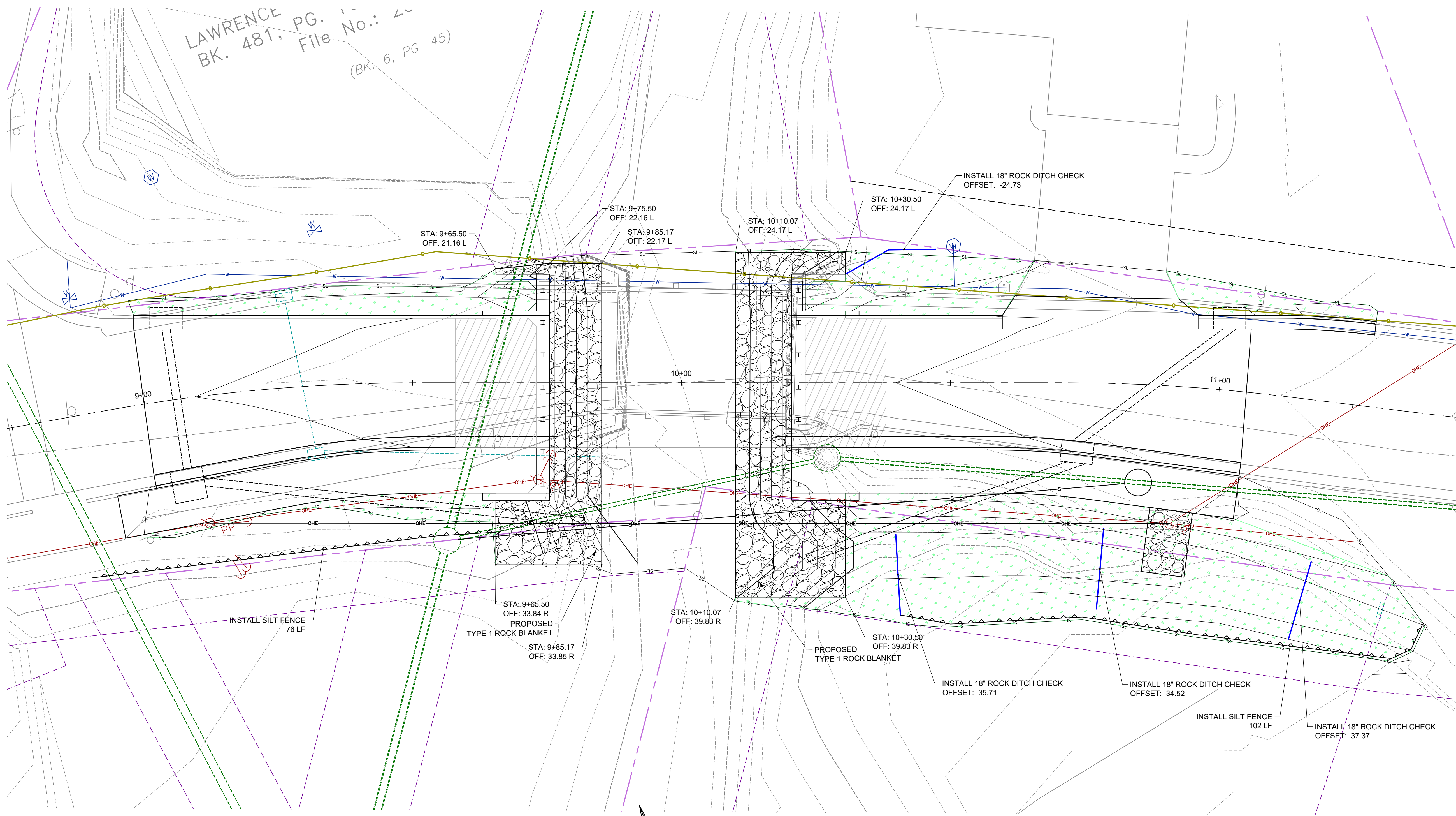
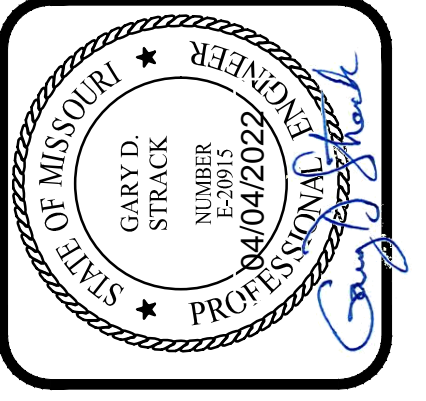


LAWRENCE PG. 1-
BK. 481, File No.: 2-
(BK. 6, PG. 45)



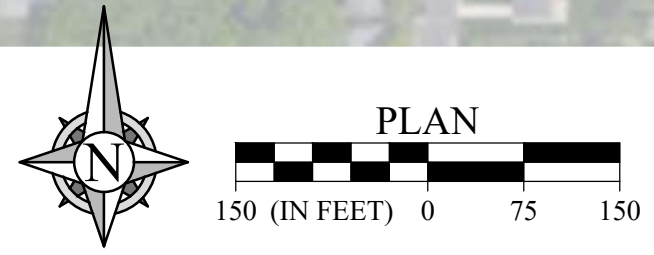
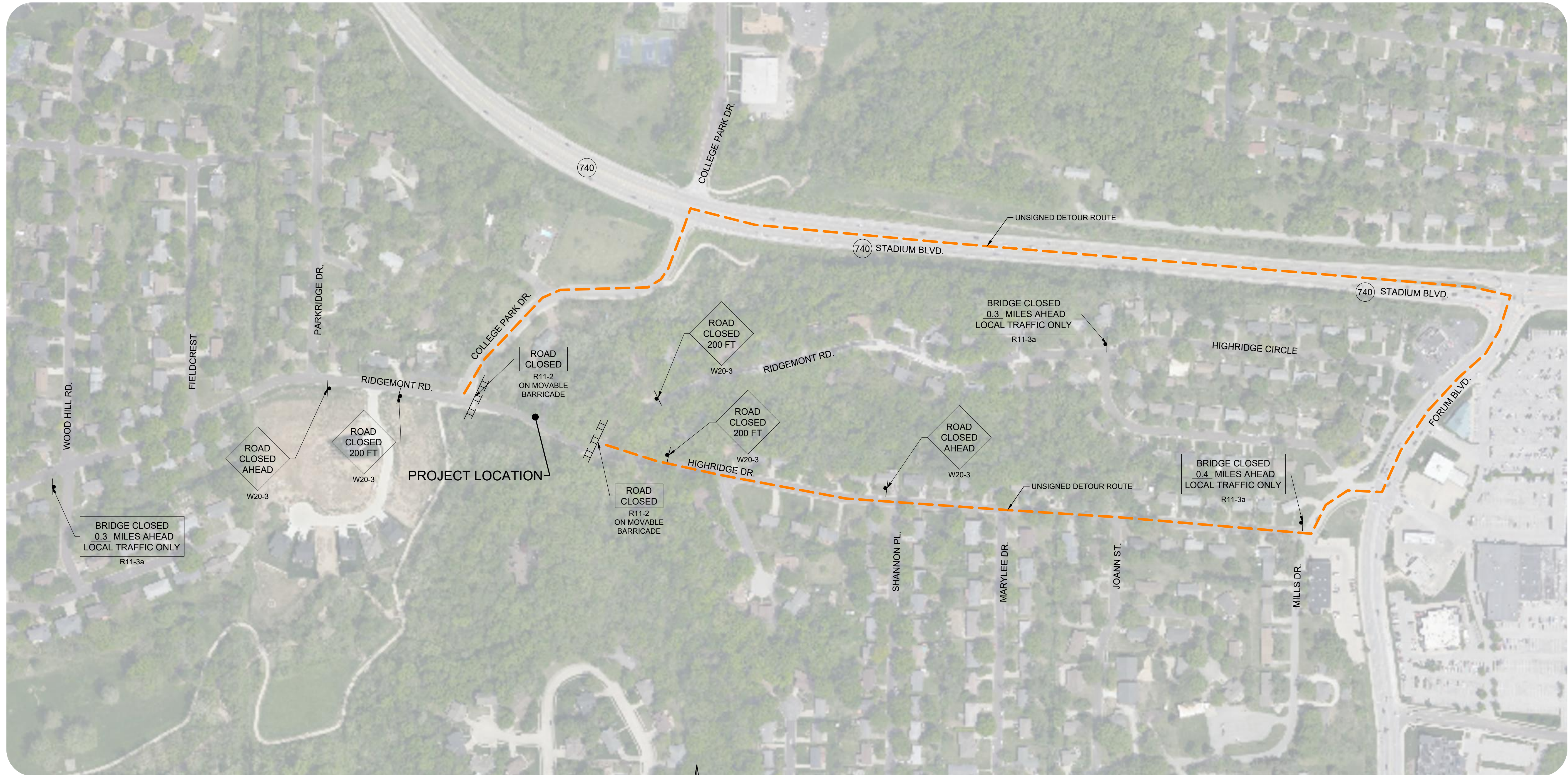
QUANTITIES:
Seeding: 0.1 Acres
Silt Fence: 114 LF
Rock Ditch Check: 63 LF



REVISIONS		DRAWING INFO.			
NO.	DESCRIPTION	BY	DATE	FIELD BY:	FIELD BOOK:
				ND/CAW	20NC40008
				DRAWN BY:	
				CHECK BY:	
				DATE:	
				JOB NUMBER:	

EROSION CONTROL
RIDGEMONT DR. BRIDGE OVER
COUNTRY HOUSE BRANCH
SEC. 22 T48N, R13W
CITY OF COLUMBIA, MISSOURI

SHEET NUMBER
4
OF **28**

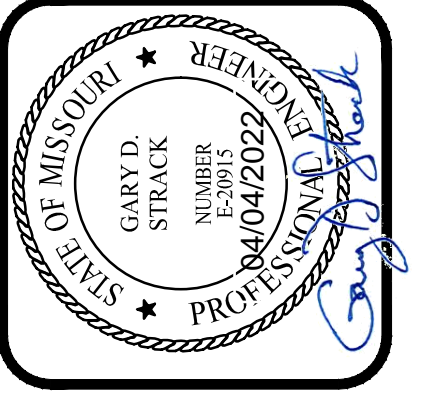


NOTES:
 TRAFFIC CONTROL SHALL COMPLY WITH THE MUTCD (2009) WITH REVISION NUMBERS 1 AND 2, AND WITH MISSOURI STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (2020)
 TRAFFIC CONTROL LAYOUT, QUANTITIES AND ITEMS MAY BE ADJUSTED BY THE ENGINEER IF DEEMED NECESSARY AND WILL BE THE RESPONSIBILITY OF THE CONTRACTOR.

LEGEND
 - - - - - UNSIGNED LOCAL DETOUR
 [Symbol] SIGN
 [Symbol] MOVABLE BARRICADE

SIGNING QUANTITIES				
SIGN	SIZE (IN.)	AREA (S.F.)	QTY.	TOTAL AREA (S.F.)
R11-2	48x30	10.00	2	20.00
R11-3a	60x30	12.50	2	25.00
W20-1	48x48	16.00	1	16.00
TOTAL				61.00

MOVABLE BARRICADE	6
--------------------------	----------

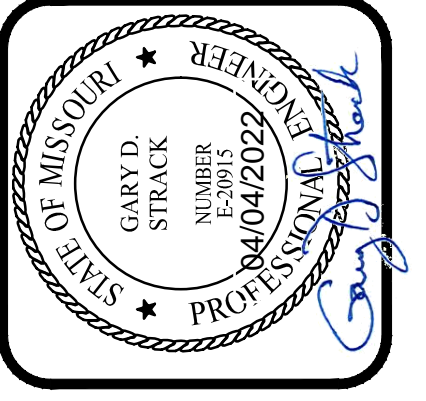
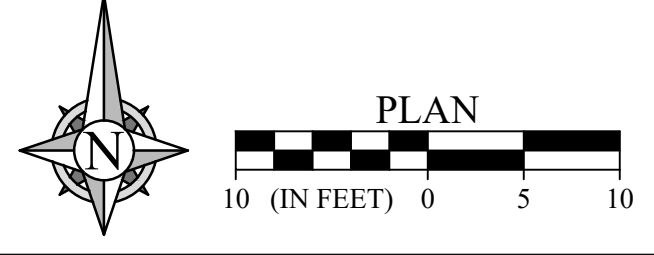
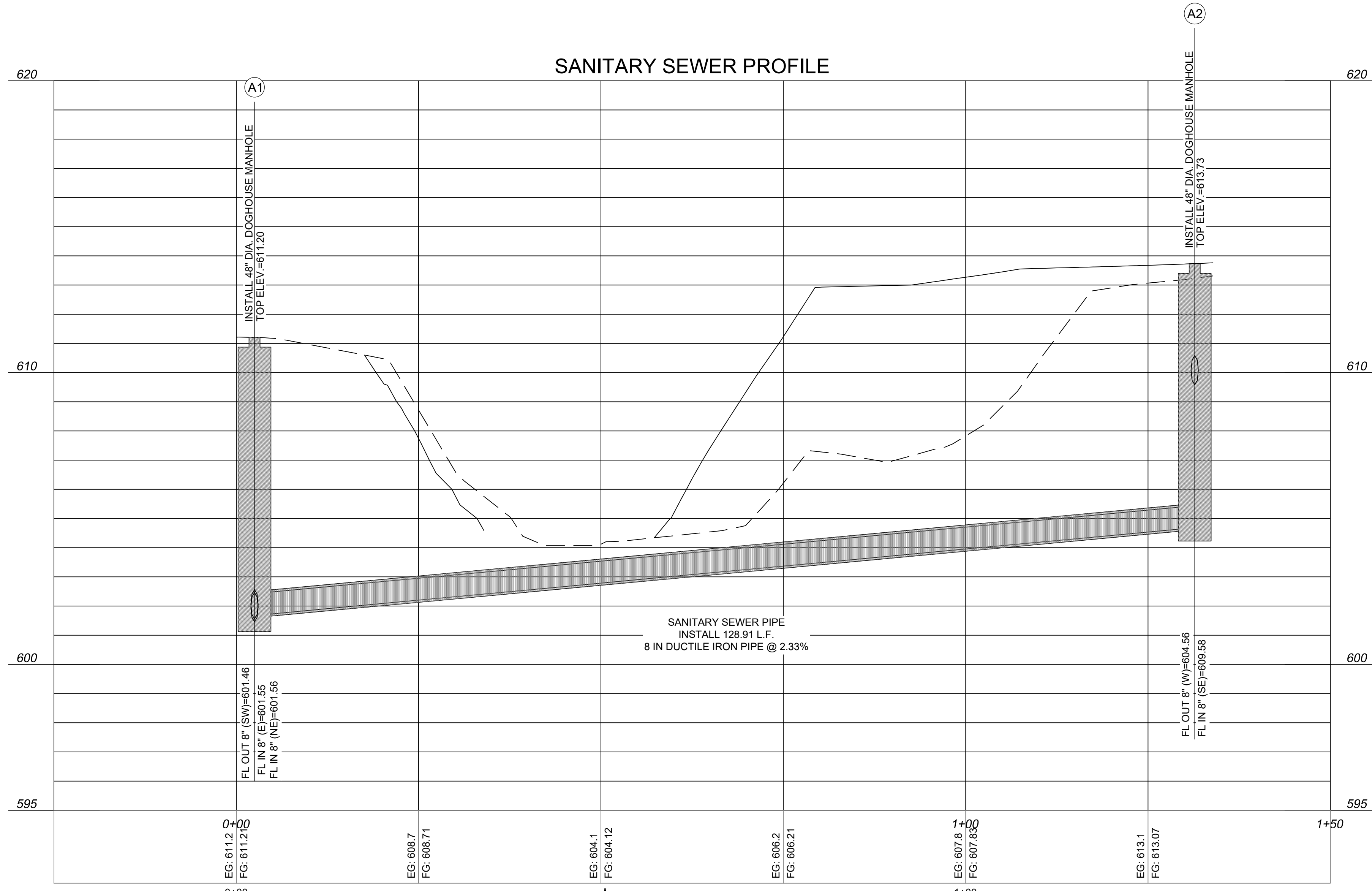
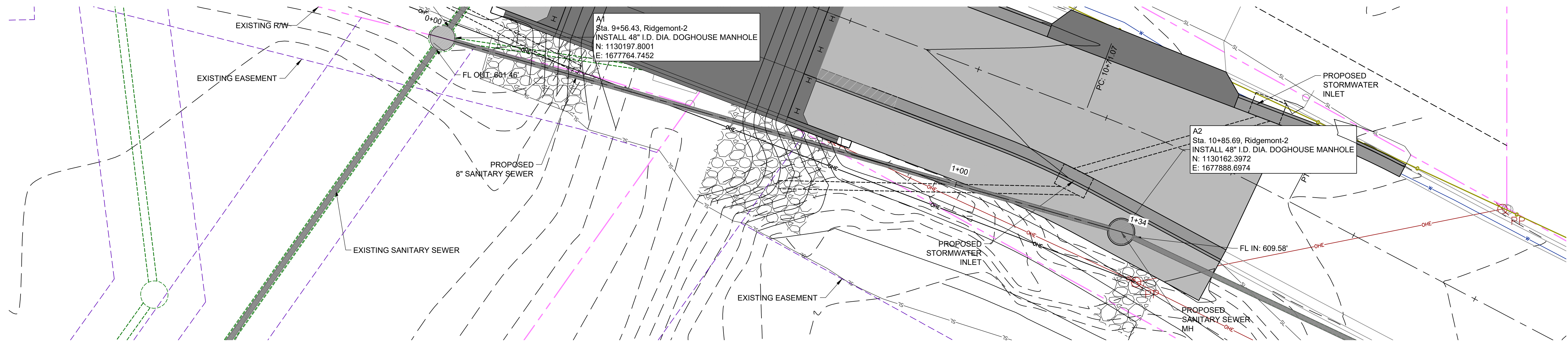


REVISIONS		DRAWING INFO.			
NO.	DESCRIPTION	BY	DATE	FIELD BY:	FIELD BOOK:
				ND/CAW	
				DRAWN BY:	
				CHECK BY:	
				DATE:	
				JOB NUMBER:	

TRAFFIC CONTROL
 RIDGEMONT DR. BRIDGE OVER
 COUNTRY HOUSE BRANCH
 SEC. 22 T48N, R13W
 CITY OF COLUMBIA, MISSOURI

SHEET NUMBER
5
 OF **28**

© Shared drawing: ND/CAW/MS/CS/0008 Ridgemont Bridge/CAD/Plan/PL Traffic Control Plan



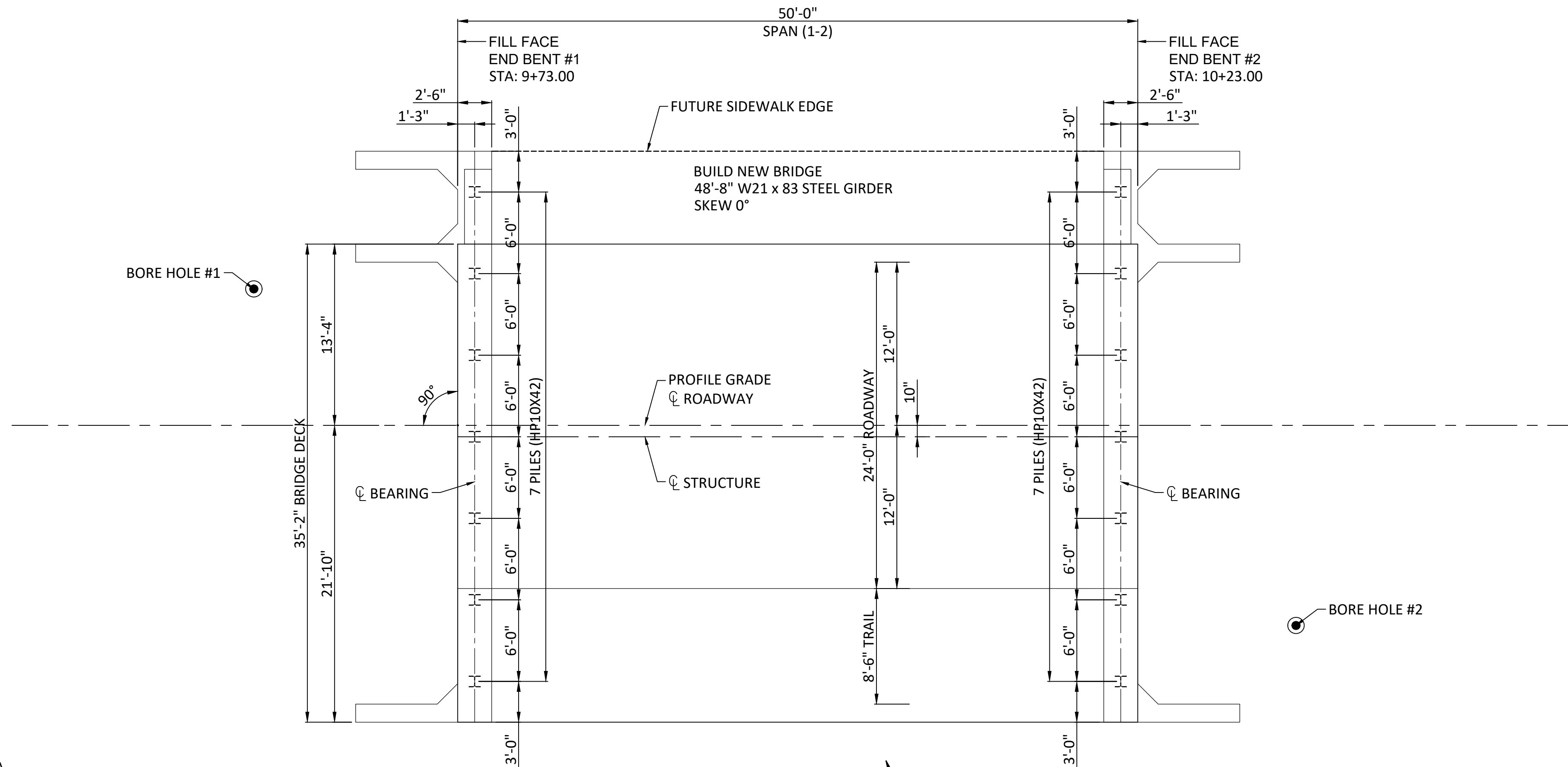
REVISIONS		DRAWING INFO.	
NO.	DESCRIPTION	BY	DATE

FIELD BY:	ND/CAW
DRAWN BY:	GDS
CHECK BY:	10/21/2021
DATE:	
FIELD BOOK:	2014C4008
JOB NUMBER:	

SANITARY PLAN & PROFILES
 RIDGEMONT DR. BRIDGE OVER
 COUNTRY HOUSE BRANCH
 SEC. 22 T48N, R13W
 CITY OF COLUMBIA, MISSOURI

SHEET NUMBER
8
 OF 28

© Shared drive/193_ND/Columbia/C4008_Ridgemont Bridge/CAD/Plan/18_Sanitary Sewer Plan



BRIDGE PLAN

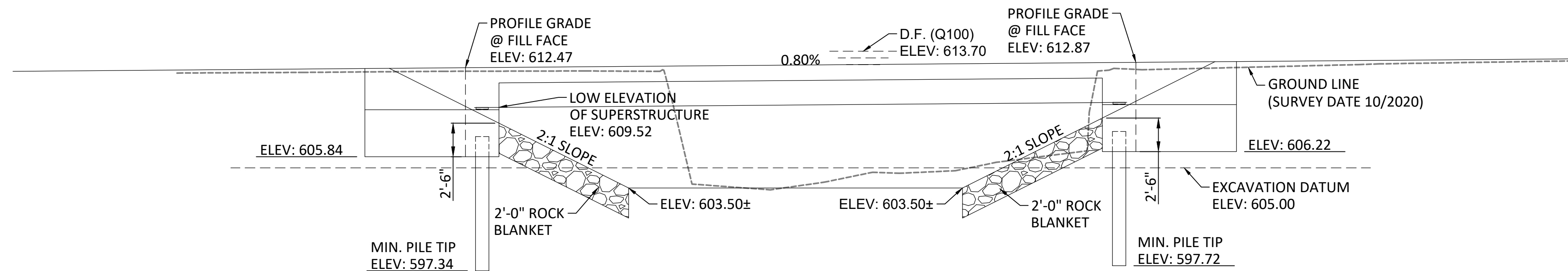


● INDICATES LOCATION OF BORINGS.

NOTICE AND DISCLAIMER REGARDING BORING LOG DATA

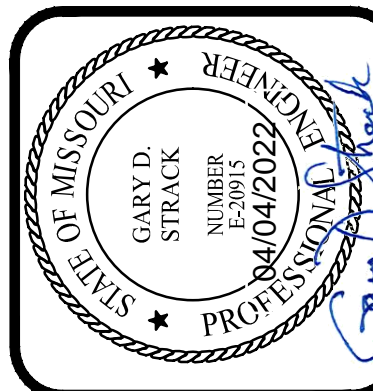
THE LOCATIONS OF ALL SUBSURFACE BORINGS FOR THIS STRUCTURE ARE SHOWN ON THE BRIDGE PLAN SHEET(S) FOR THIS STRUCTURE. BORING DATA FOR ALL LOCATIONS IS SHOWN ON SHEET NO. B2. THE BORING DATA FOR ALL LOCATIONS INDICATED, AS WELL AS ANY OTHER BORING LOGS OR OTHER FACTUAL RECORDS OF SUBSURFACE DATA AND INVESTIGATIONS PERFORMED BY THE DEPARTMENT FOR THE DESIGN OF THE PROJECT, WILL BE PROVIDED IN THE BRIDGE ELECTRONIC DELIVERABLE FILE OR WILL BE AVAILABLE FROM THE PROJECT CONTACT UPON WRITTEN REQUEST. NO GREATER SIGNIFICANCE OR WEIGHT SHOULD BE GIVEN TO THE BORING DATA DEPICTED ON THE PLAN SHEETS THAN IS SUBSURFACE DATA AVAILABLE FROM THE DISTRICT OR ELSEWHERE.

THE COMMISSION DOES NOT REPRESENT OR WARRANT THAT ANY SUCH BORING DATA ACCURATELY DEPICTS THE CONDITIONS TO BE ENCOUNTERED IN CONSTRUCTING THIS PROJECT. A CONTRACTOR ASSUMES ALL RISKS IT MAY ENCOUNTER IN BASING ITS BID PRICES, TIME OR SCHEDULE OF PERFORMANCE ON THE BORING DATA DEPICTED HERE OR THOSE AVAILABLE FROM THE DISTRICT, OR ON ANY OTHER DOCUMENTATION NOT EXPRESSLY WARRANTED, WHICH THE CONTRACTOR MAY OBTAIN FROM THE COMMISSION.



BRIDGE PROFILE

HYDROLOGIC DATA	
DRAINAGE AREA	= 1.05 SQ. MI.
DESIGN FLOOD FREQUENCY	= 100 YEARS
DESIGN FLOOD DISCHARGE	= 3,486 CFS
DESIGN FLOOD (D.F.) ELEVATION	= 613.70
BASE FLOOD (100-YEAR)	
BASE FLOOD ELEVATION	= 613.70
BASE FLOOD DISCHARGE	= 3,486 CFS
ESTIMATED BACKWATER	= 1.50 FT
FREEBOARD (10-YEAR)	
FREEBOARD	= -0.25 FT (HIGH SIDE)
ROADWAY OVERTOPPING	
OVERTOPPING FLOOD DISCHARGE	= 2,220 CFS
OVERTOPPING FLOOD FREQUENCY	= 10 YEARS
OVERTOPPING FLOOD ELEVATION	= 611.95

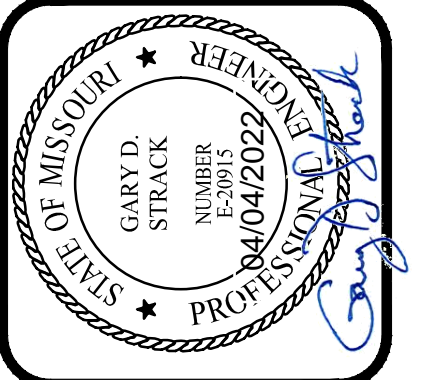


DRAWING INFO.	
FIELD BY:	CAW
DRAWN BY:	GDS
CHECK BY:	10/29/2020
DATE:	FIELD BOOK:
FIELD NO.:	JOB NUMBER:
NO.	2022040008

BRIDGE PLAN & PROFILE
 RIDGEMONT DR. BRIDGE OVER
 COUNTY HOUSE BRANCH
 SEC. 22 T48N, R13W
 CITY OF COLUMBIA, MISSOURI

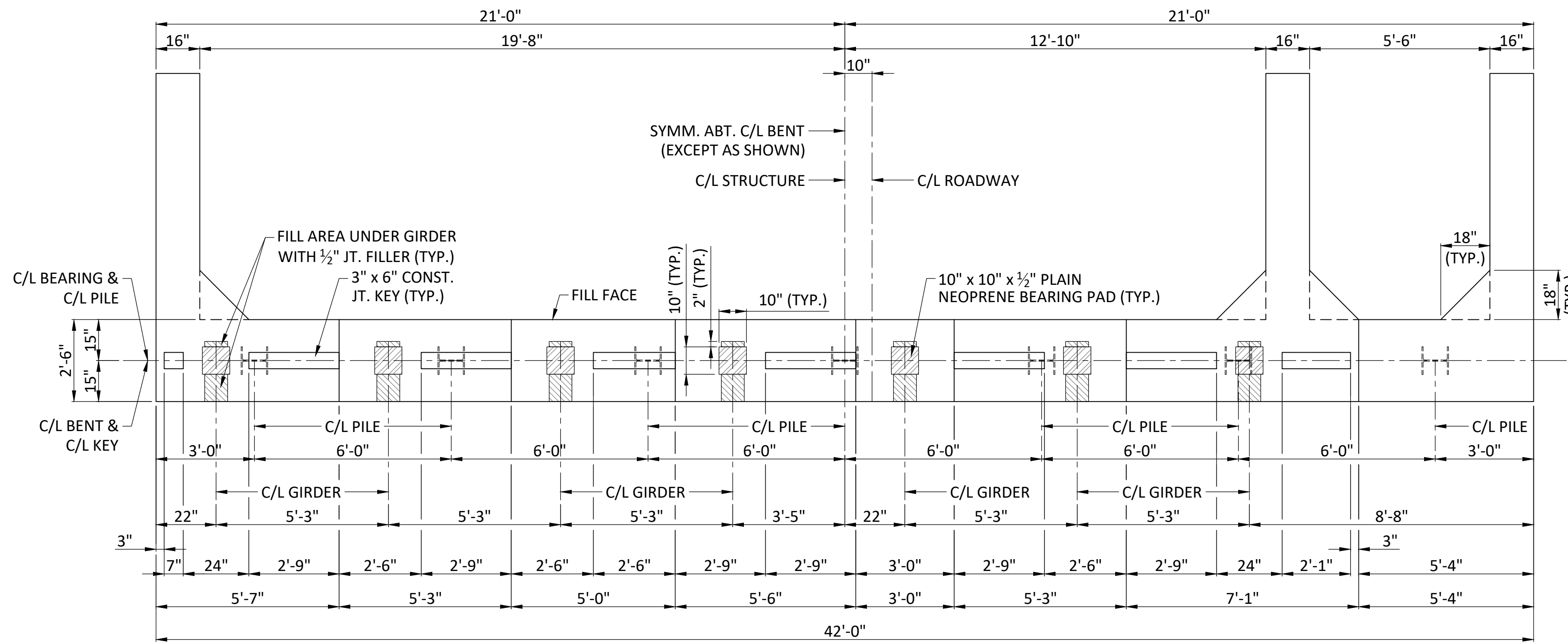
SHEET NUMBER
10
 OF 28

© 2022 Anderson Engineering, Inc. All Rights Reserved. Bridge Plan & Profile

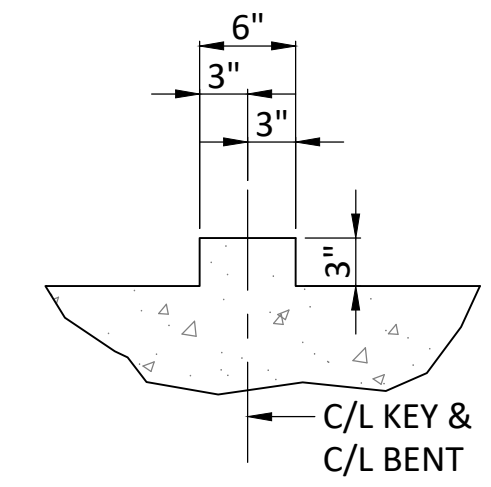


REVISIONS		DRAWING INFO.	
NO.	DESCRIPTION	BY	DATE

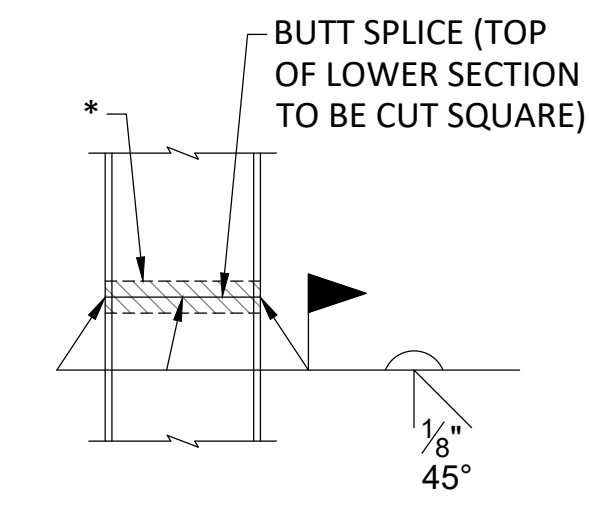
DRAWN BY:	CAW	FIELD NO.:	2018042022
CHECKED BY:	GDS	FIELD BOOK:	
DATE:	10/29/2020	JOB NUMBER:	2018CA0008



PLAN OF BEAM

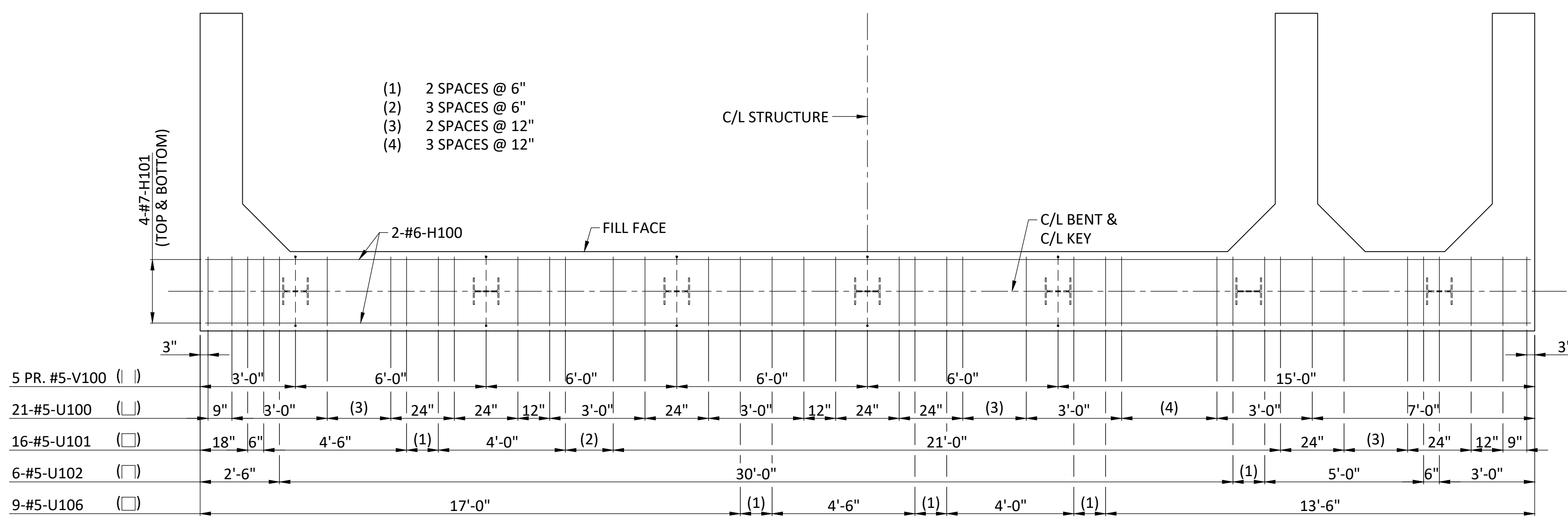


SECTION THRU KEY



STEEL PILE SPlice
(IF REQUIRED)

* GALVANIZING MATERIAL SHALL BE OMITTED OR REMOVED ONE INCH CLEAR OF WELD LOCATIONS IN ACCORDANCE WITH SEC 702.



PLAN OF BEAM SHOWING REINFORCEMENT
KEYS NOT SHOWN FOR CLARITY.

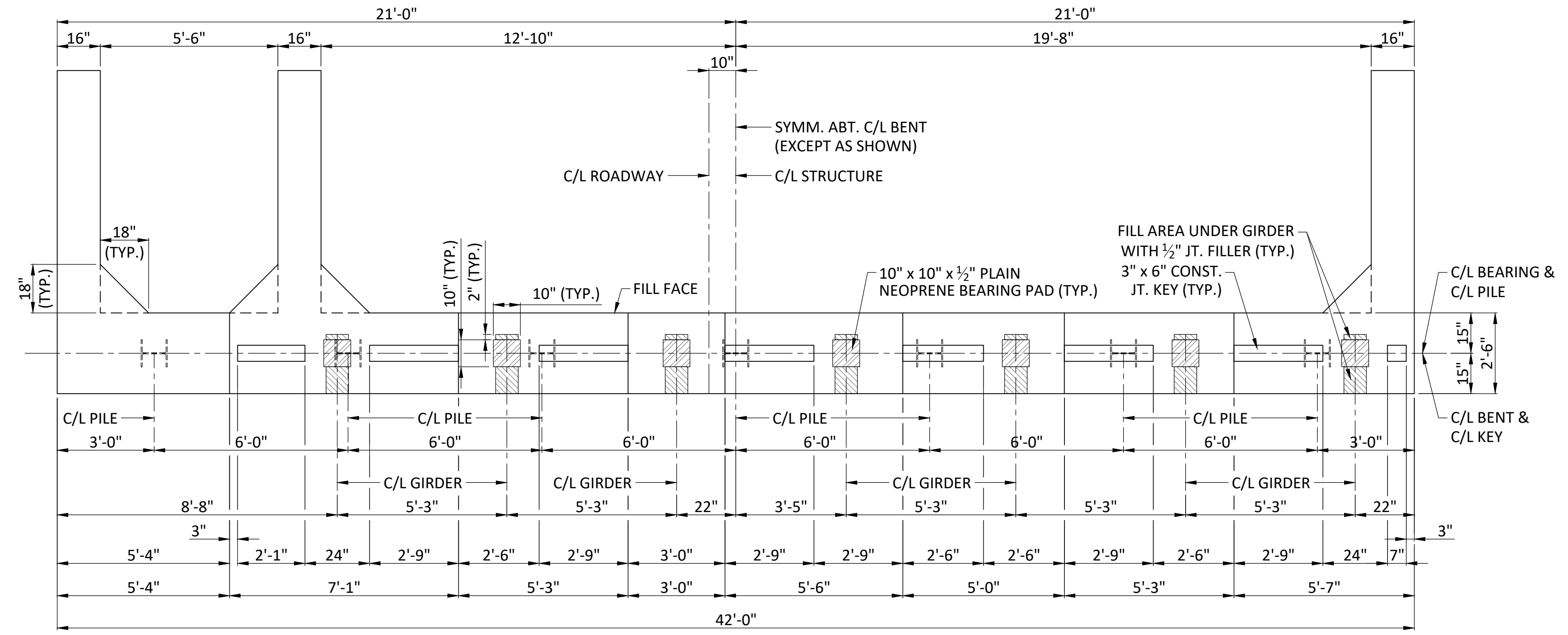
GENERAL NOTES:
WORK THIS SHEET WITH SHEETS NO. 12 & 13.
ALL U BARS AND PAIRS OF V BARS SHALL BE PLACED PARALLEL TO CENTERLINE OF ROADWAY.
REINFORCING STEEL SHALL BE SHIFTED TO CLEAR PILES.
U BARS SHALL CLEAR PILES BY AT LEAST 1 1/2 INCHES.

ITEM	QUANTITY
CLASS 1 EXCAVATION	CU. YARD 43.0
GALVANIZED STRUCTURAL STEEL PILES (12 IN.)	LINEAR FOOT 70
PRE-BORE FOR PILING	LINEAR FOOT 59.5
PILE POINT REINFORCEMENT	EACH 7
CLASS B CONCRETE (SUBSTRUCTURE)	CU. YARD 18.8

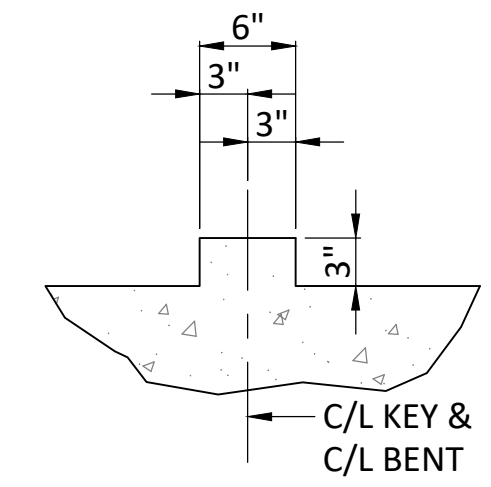
THESE QUANTITIES ARE INCLUDED IN THE ESTIMATED QUANTITIES TABLE ON SHEET NO. 9

END BENT 1
RIDGEMONT DR. BRIDGE OVER
COUNTY HOUSE BRANCH
SEC. 22 T48N, R13W
CITY OF COLUMBIA, MISSOURI

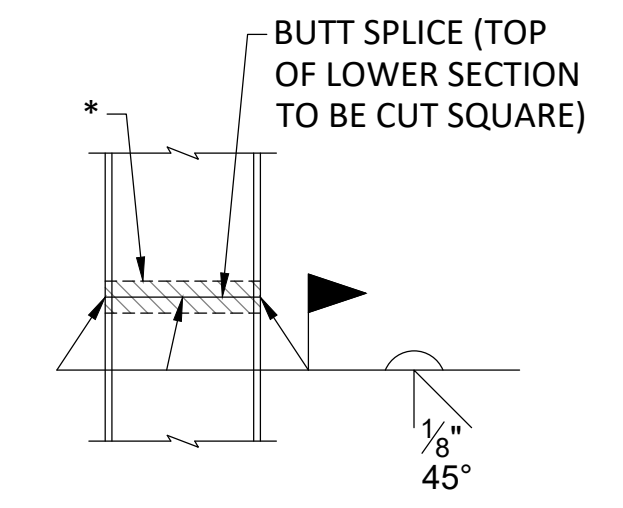
SHEET NUMBER
11
OF **28**



PLAN OF BEAM

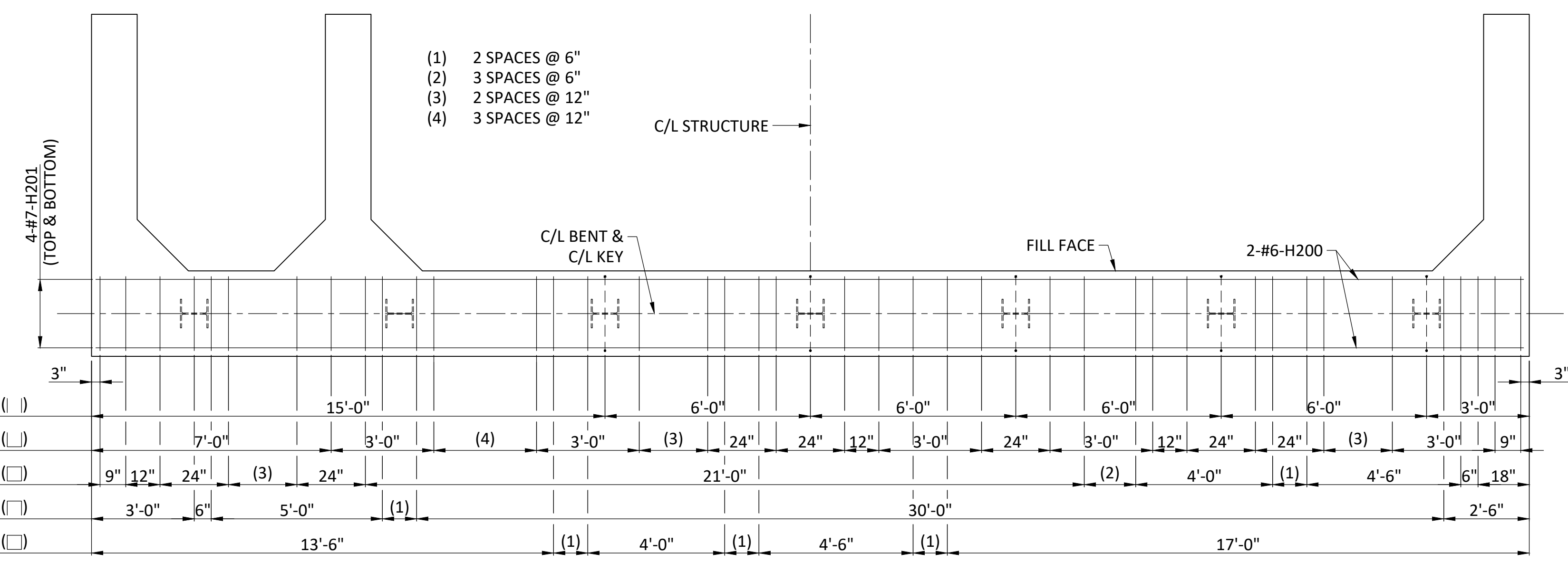


SECTION THRU KEY



STEEL PILE SPlice
(IF REQUIRED)

* GALVANIZING MATERIAL SHALL BE OMITTED OR REMOVED ONE INCH CLEAR OF WELD LOCATIONS IN ACCORDANCE WITH SEC 702.



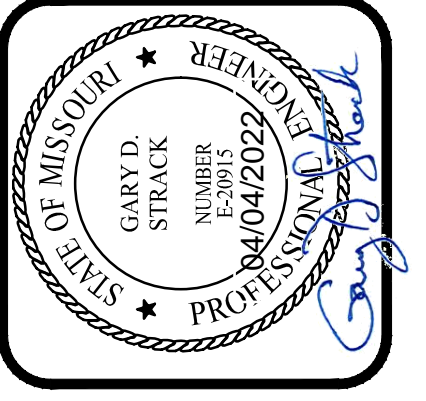
PLAN OF BEAM SHOWING REINFORCEMENT
KEYS NOT SHOWN FOR CLARITY.

GENERAL NOTES:

WORK THIS SHEET WITH SHEETS NO. 15 & 16.
ALL U BARS AND PAIRS OF V BARS SHALL BE PLACED PARALLEL TO CENTERLINE OF ROADWAY.
REINFORCING STEEL SHALL BE SHIFTED TO CLEAR PILES.
U BARS SHALL CLEAR PILES BY AT LEAST 1 1/2 INCHES.

SUBSTRUCTURE QUANTITY TABLE FOR BENT NO. 2		
ITEM		QUANTITY
CLASS 1 EXCAVATION	CU. YARD	41.8
GALVANIZED STRUCTURAL STEEL PILES (12 IN.)	LINEAR FOOT	70
PRE-BORE FOR PILING	LINEAR FOOT	59.5
PILE POINT REINFORCEMENT	EACH	7
CLASS B CONCRETE (SUBSTRUCTURE)	CU. YARD	18.8

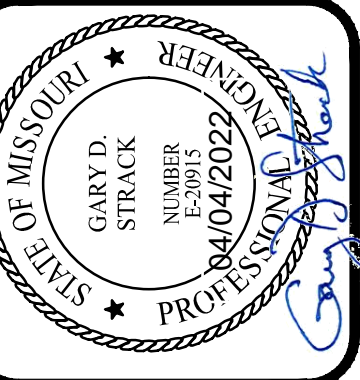
THESE QUANTITIES ARE INCLUDED IN THE ESTIMATED QUANTITIES TABLE ON SHEET NO. 9



DRAWING INFO.		REVISIONS	
NO.	DESCRIPTION	DATE	BY
FIELD BY:	CAW		
DRAWN BY:	GDS		
CHECK BY:			
DATE:	10/29/2020		
FIELD BOOK:			
JOB NUMBER:	20NC40008		

END BENT 2
RIDGEMONT DR. BRIDGE OVER
COUNTY HOUSE BRANCH
SEC. 22 T48N, R13W
CITY OF COLUMBIA, MISSOURI

SHEET NUMBER
14
OF **28**

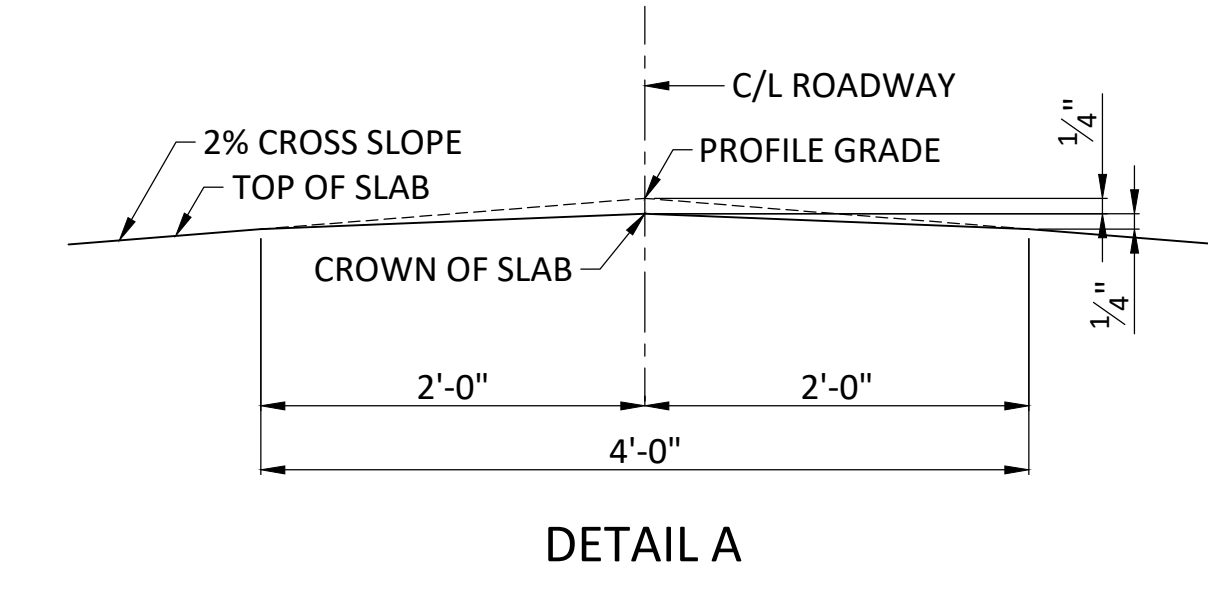


DRAWING INFO.	
NO.	DESCRIPTION
BY	DATE
FIELD BY:	CAW
DRAWN BY:	GDS
CHECK BY:	10/29/2020
DATE:	FIELD BOOK:
DATE:	JOB NUMBER:
DATE:	20NC40008

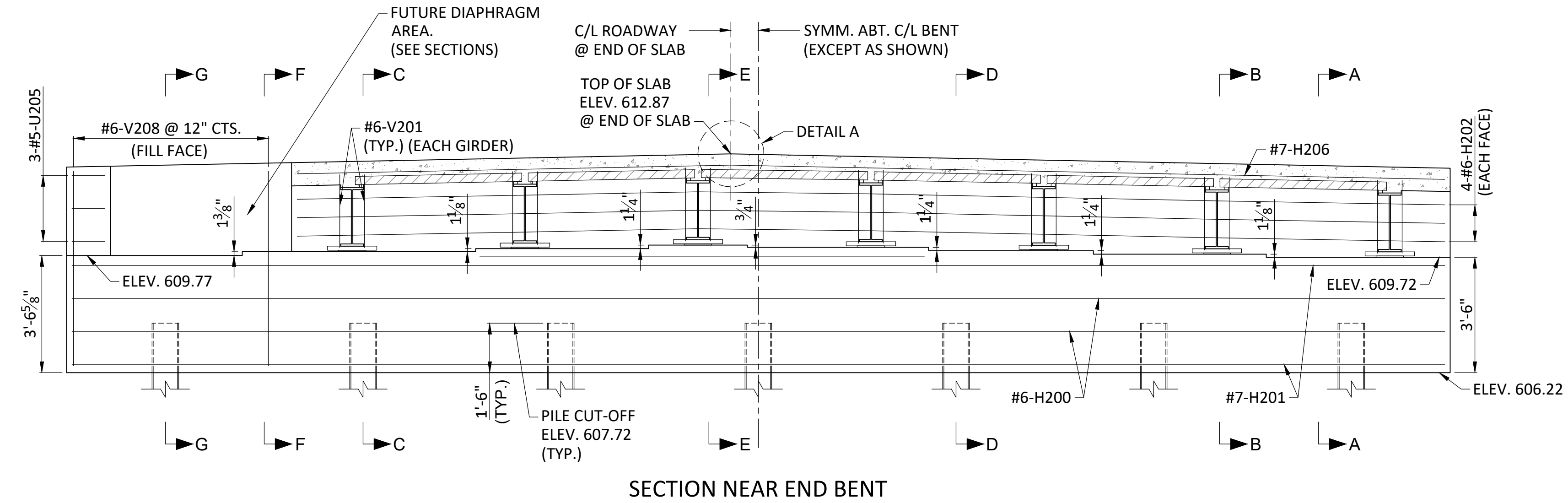
END BENT 2
 RIDGEMONT DR. BRIDGE OVER
 COUNTY HOUSE BRANCH
 SEC. 22 T48N, R13W
 CITY OF COLUMBIA, MISSOURI

SHEET NUMBER
15
 OF 28

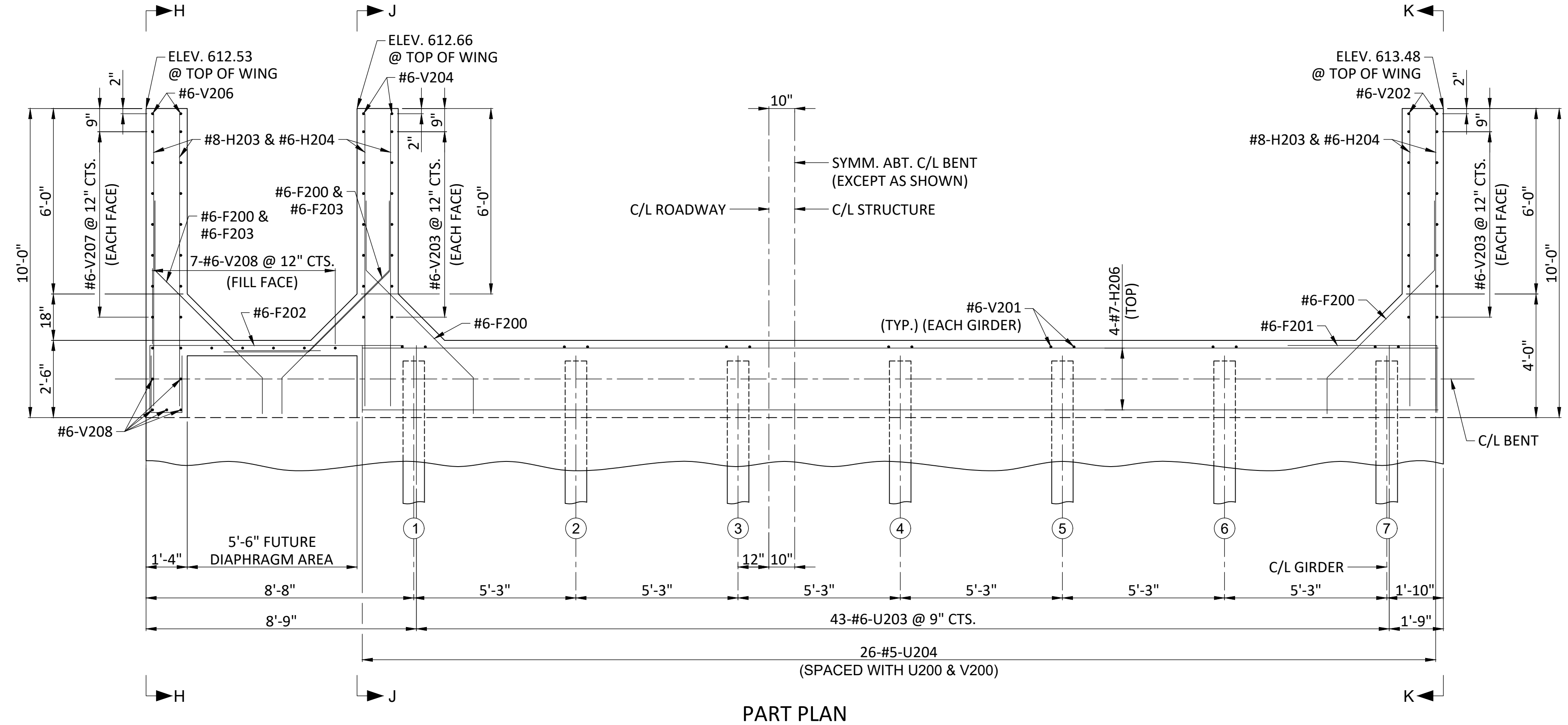
© 2019 Anderson Engineering, Inc. All Rights Reserved.



DETAIL A

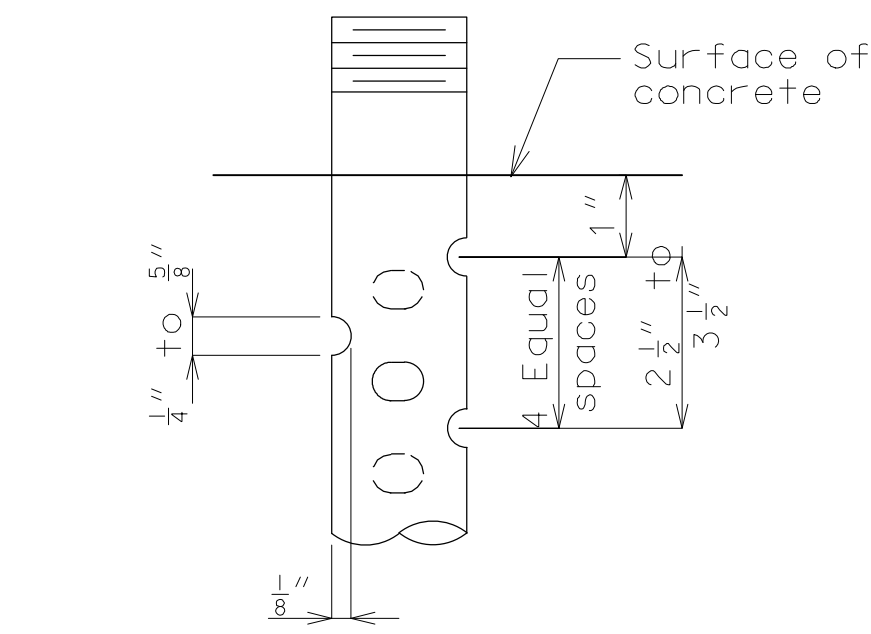


SECTION NEAR END BENT

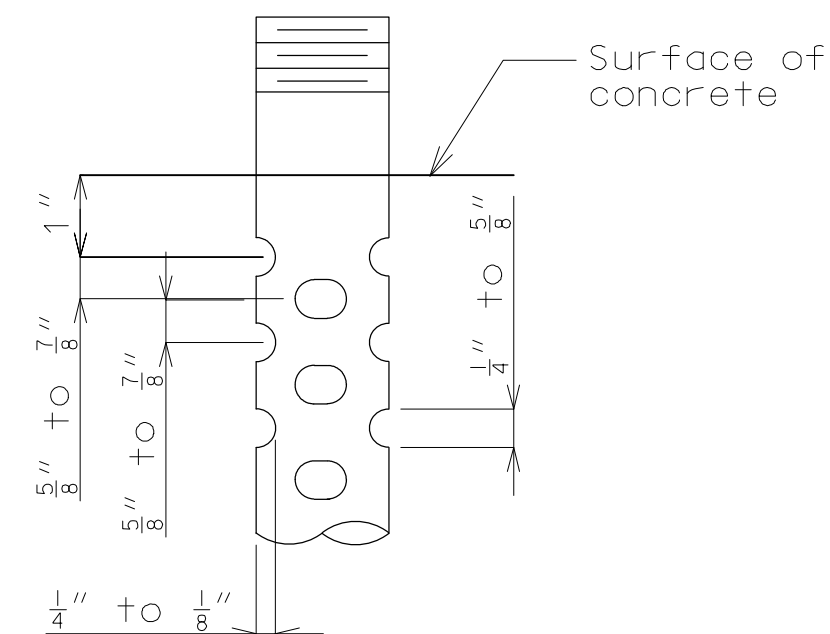


PART PLAN

GENERAL NOTES:
 WORK THIS SHEET WITH SHEETS NO. 14 & 16.
 FOR SECTIONS AND ELEVATIONS SEE SHEET NO. 16
 THE #6-F200 BARS SHALL BE BENT IN THE FIELD TO CLEAR GIRDERS.
 ALL U BARS AND PAIRS OF V BARS SHALL BE PLACED PARALLEL TO CENTERLINE OF ROADWAY.
 ALL CONCRETE IN THE END BENT ABOVE TOP OF BEAM AND BELOW TOP OF SLAB SHALL BE CLASS B-2.

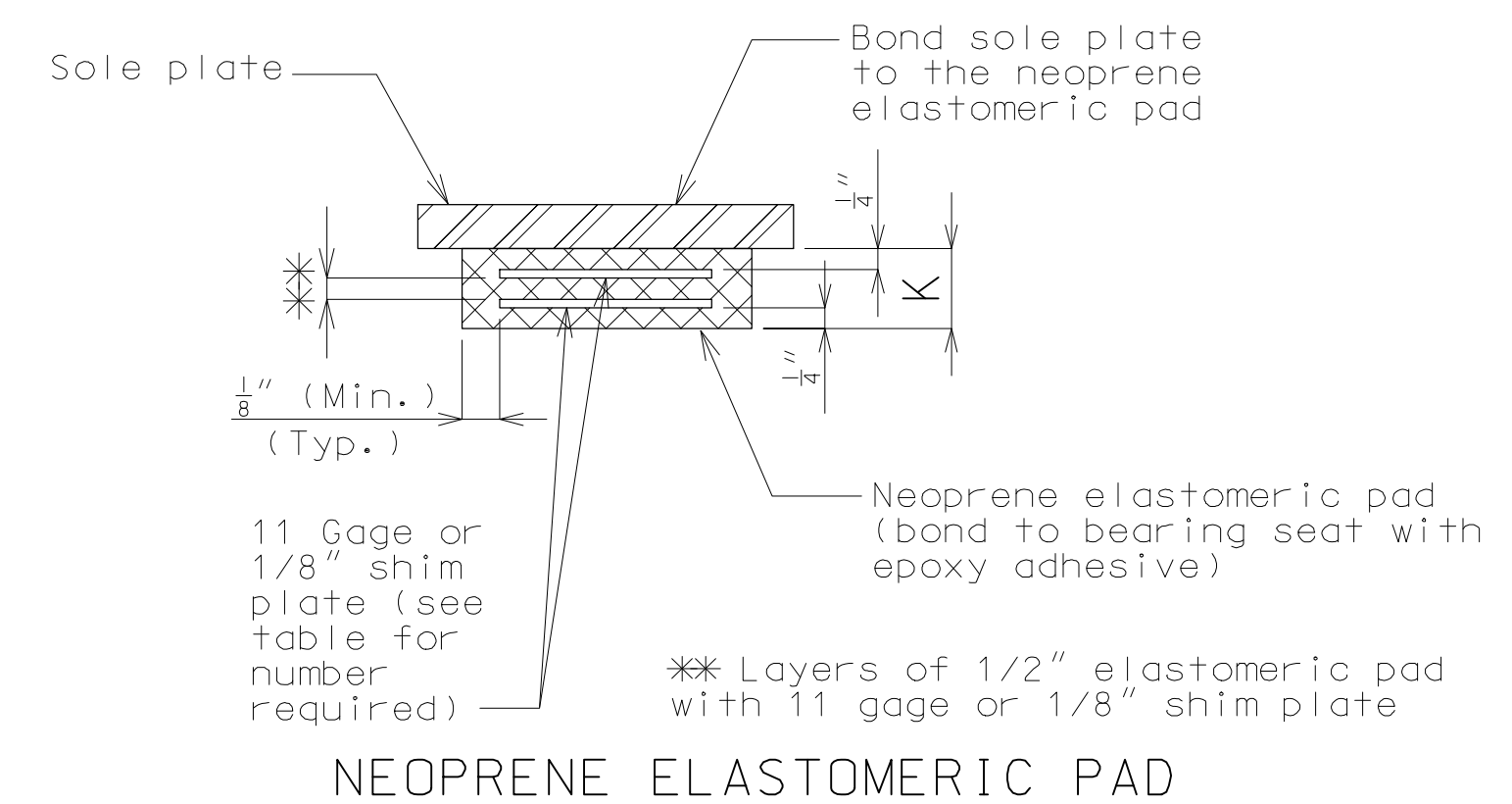
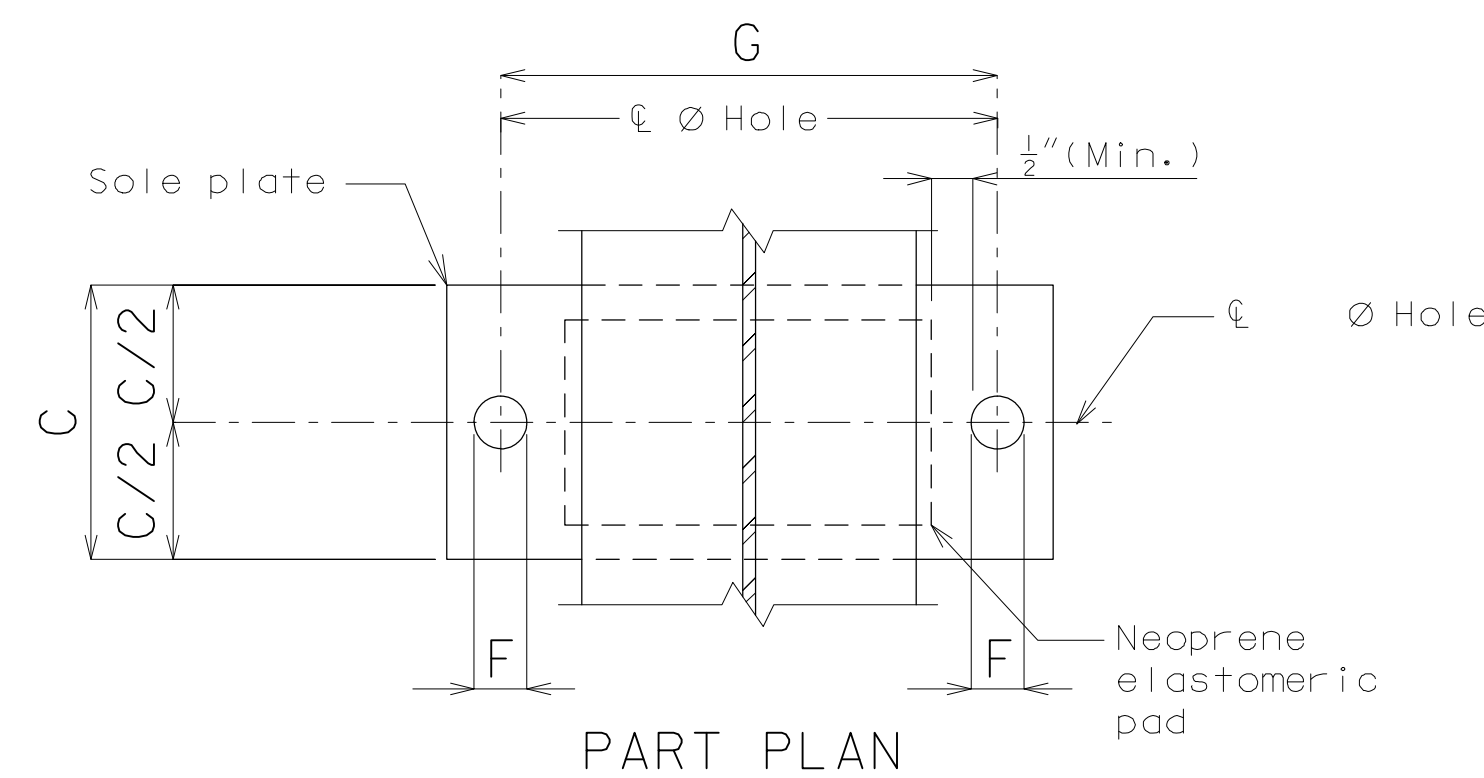
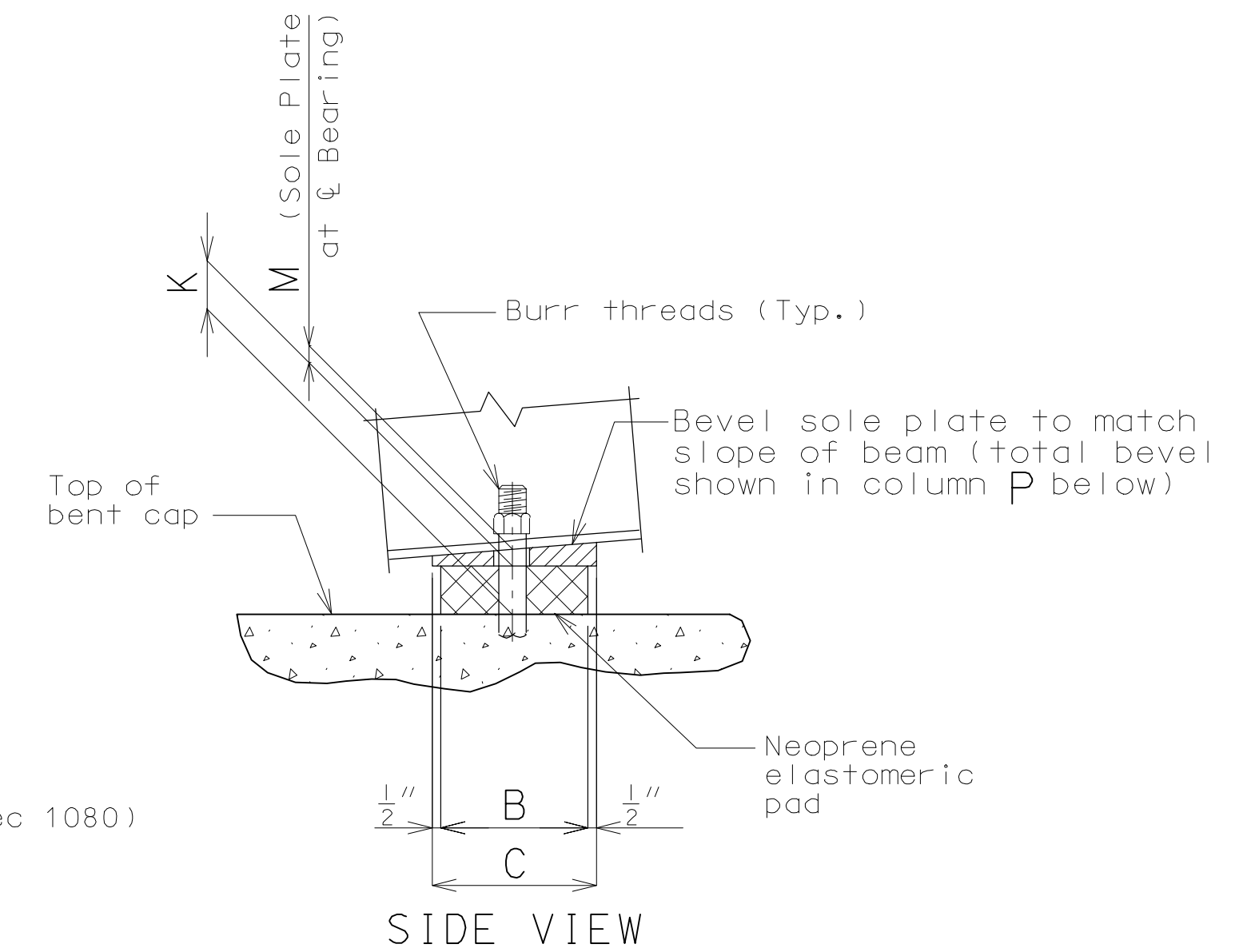
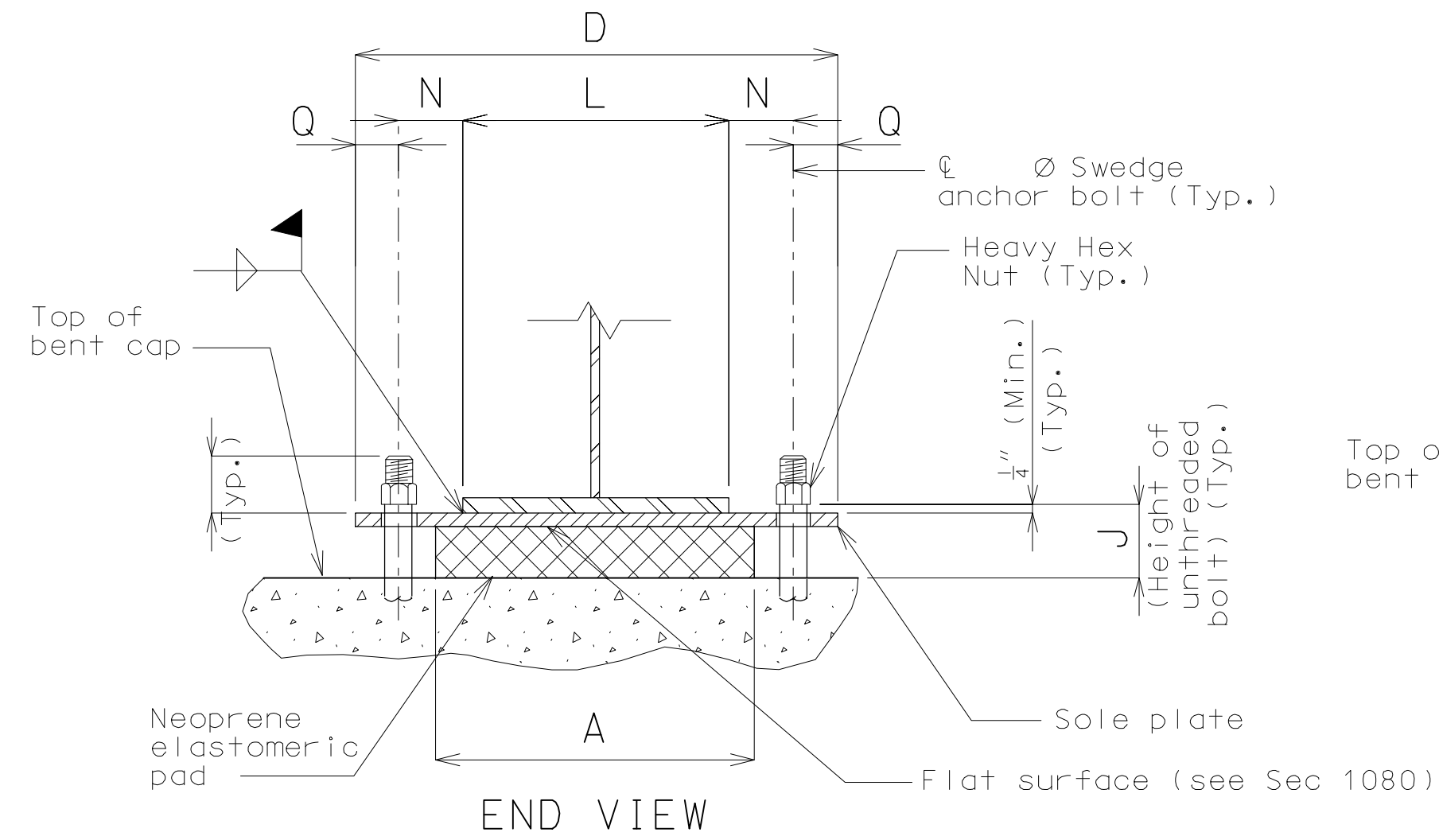


DETAIL FOR 3/4" Ø THRU 2 1/2" Ø ANCHOR BOLTS



OPTIONAL DETAIL FOR 1 3/8" Ø THRU 2 1/2" Ø ANCHOR BOLTS

SWEDGE ANCHOR BOLT DETAILS



FIXED BEARINGS														NUMBER OF SHIM PLATES *	NUMBER REQUIRED
BENT NO.	A	B	C	D	F	G	J	K	L	M	N	P	Q		
1	10"	10"	11"	18"	1 1/8"	13 1/2"	2 1/4"	1/2"	8 3/8"	1 1/2"	2 9/16"	0"	2 1/4"	0	7
2	10"	10"	11"	18"	1 1/8"	13 1/2"	2 1/4"	1/2"	8 3/8"	1 1/2"	2 9/16"	0"	2 1/4"	0	7
														TOTAL BEARINGS	14

* The required shim plate shall be placed between layers of elastomer and molded together to form an integral unit.

GENERAL NOTES:

Anchor bolts shall be 1" Ø ASTM F1554 Grade 55 swaged bolts and shall extend 15" into the concrete with ASTM A563 Grade A Heavy Hex nuts. Actual manufacturer's certified mill test reports (chemical and mechanical) shall be provided. Swedging shall be 1" less than extension into the concrete.

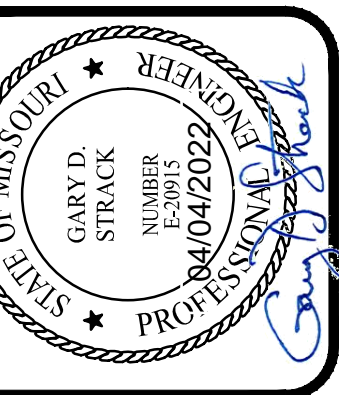
Anchor bolts and heavy hex nuts shall be coated with a minimum of two coats of inorganic zinc primer to provide a total dry film thickness of 4 mils minimum, 6 mils maximum, or galvanized in accordance with Sec 1081.

Neoprene Elastomeric Pads shall be 60 Durometer.

Structural steel for sole plate shall be ASTM A709 Grade 50W and shall be coated with a minimum of two coats of inorganic zinc primer to provide a total dry film thickness of 4 mils minimum, 6 mils maximum. The welds shall have corrosion resistance and weathering characteristics compatible with the base material.

Laminated Neoprene Bearing Pad Assembly shall be in accordance with Sec 716.

LAMINATED NEOPRENE BEARING PAD ASSEMBLY

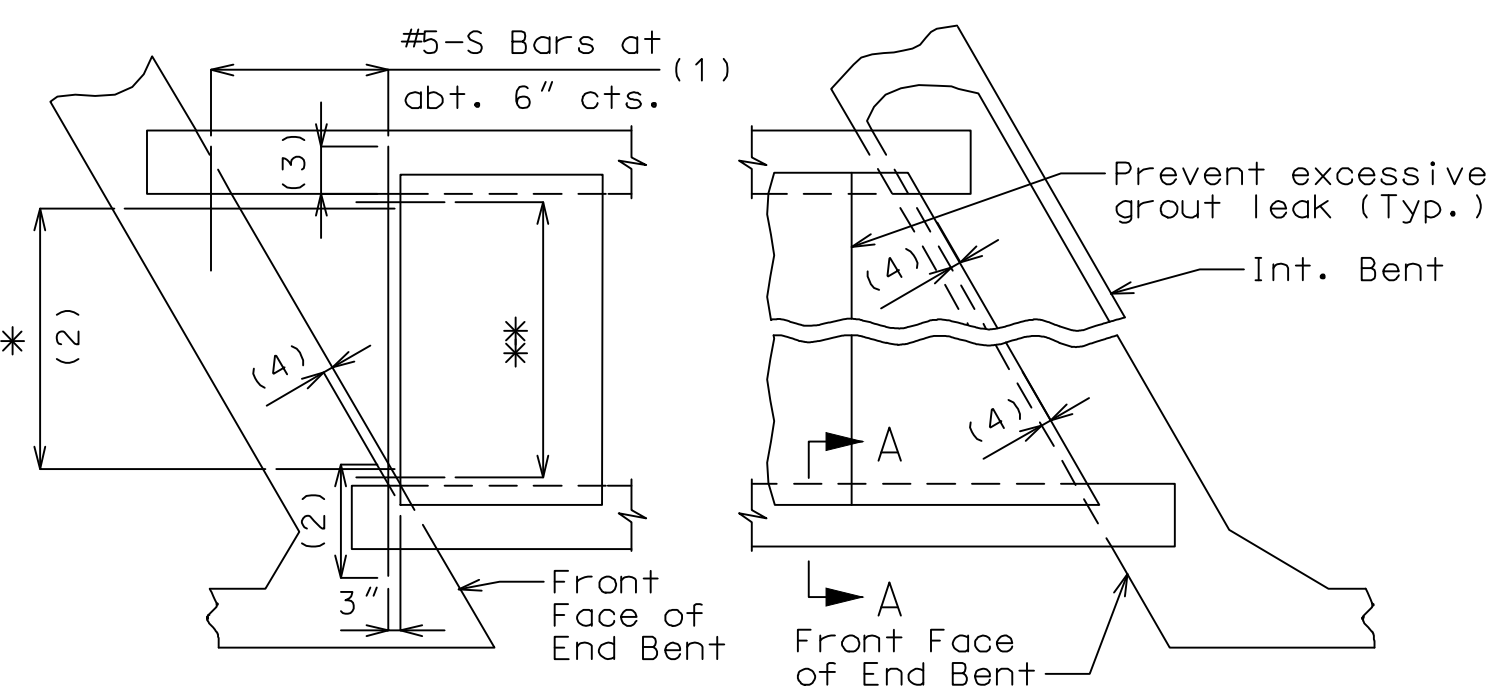


DRAWING INFO.		REVISIONS	
FIELD BY:	DATE	DESCRIPTION	NO.
CAW			
GDS			
10/29/2020			
20NC4008			
20NC4008			

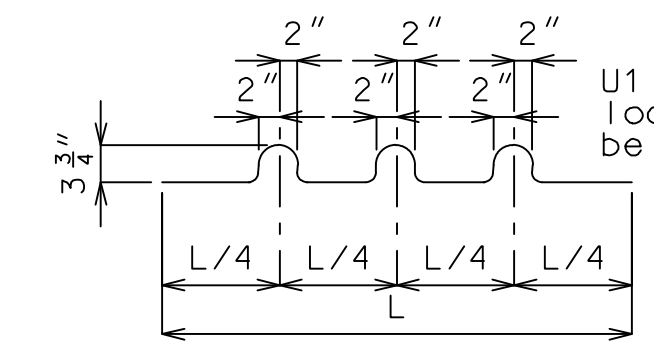
BEARING PAD ASSEMBLY
RIDGEMONT DR. BRIDGE OVER
COUNTY HOUSE BRANCH
SEC. 22 T48N, R13W
CITY OF COLUMBIA, MISSOURI

SHEET NUMBER
17
OF **28**

DRAWING INFO.		REVISIONS	
NO.	DESCRIPTION	DATE	BY
FIELD BY:	CAW		
DRAWN BY:	GDS		
CHECK BY:	10/29/2020		
DATE:	FIELD BOOK:		
	JOB NUMBER:		
	20NC40008		



SQUARED END PANELS (INTEGRAL) SKEWED END PANELS (INTEGRAL)
PLAN SHOWING PANELS PLACEMENT

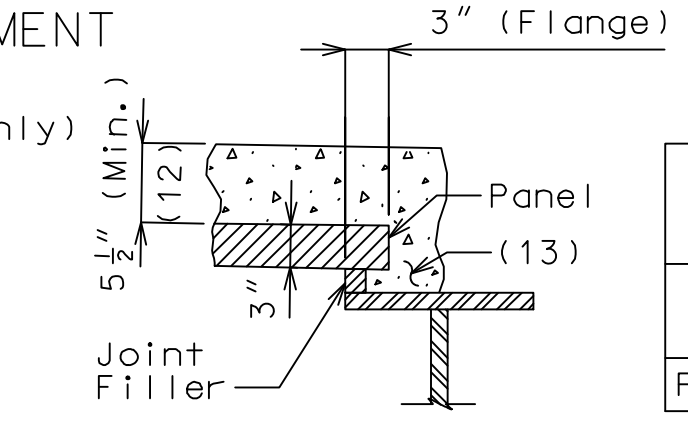


BENDING DIAGRAM FOR U1 BAR

U1 Bars may be oriented at right angles to location and spacing shown. U1 Bars shall be placed between P1 bars.

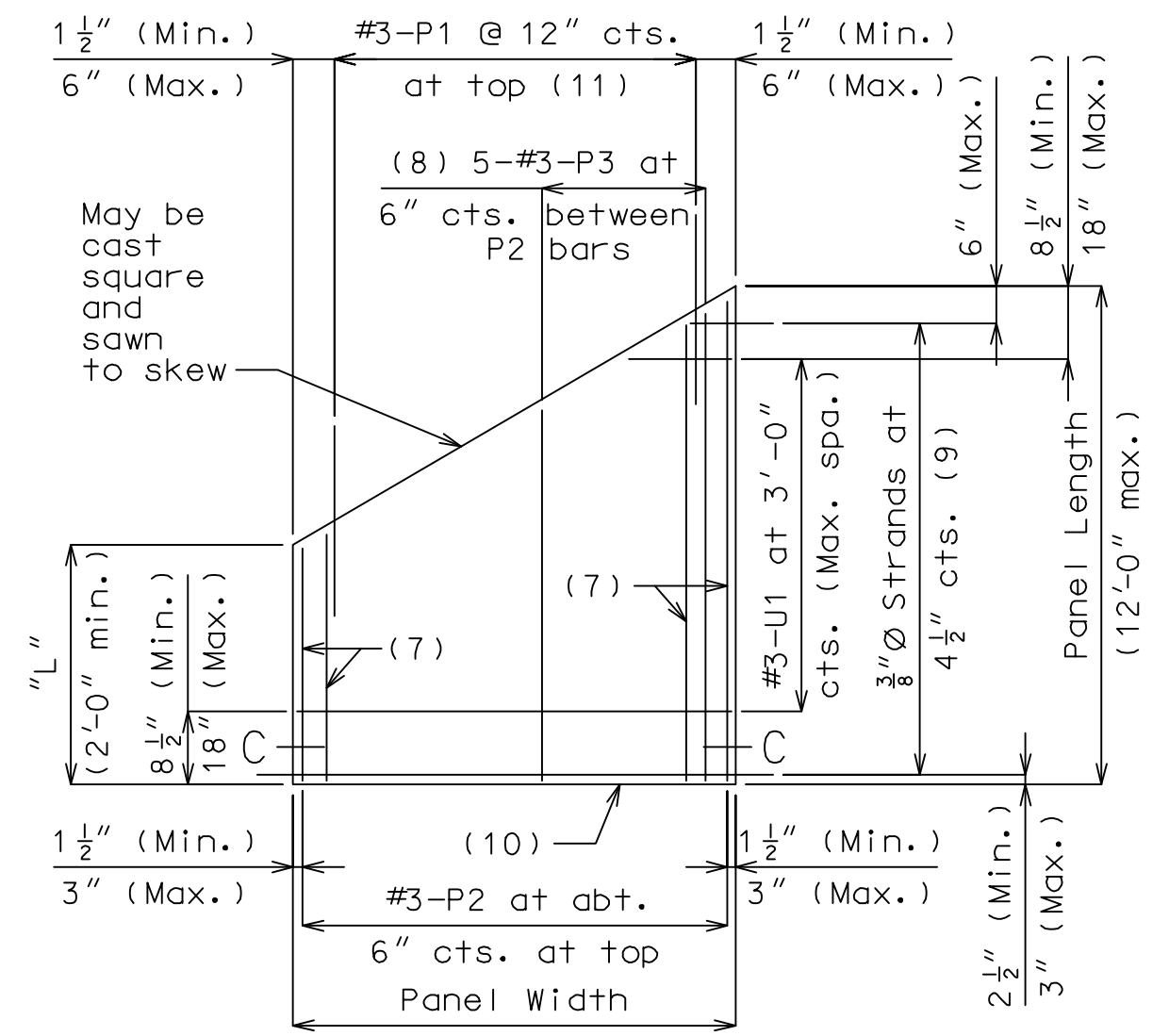
* #5-S Bars at abt. 9" cts. (1)
** #3-P1 at 12" cts. (End panels only)

Loc.	Width (W)	Height (H)
		Min. Max.
Flange	1 1/2"	1" 2"

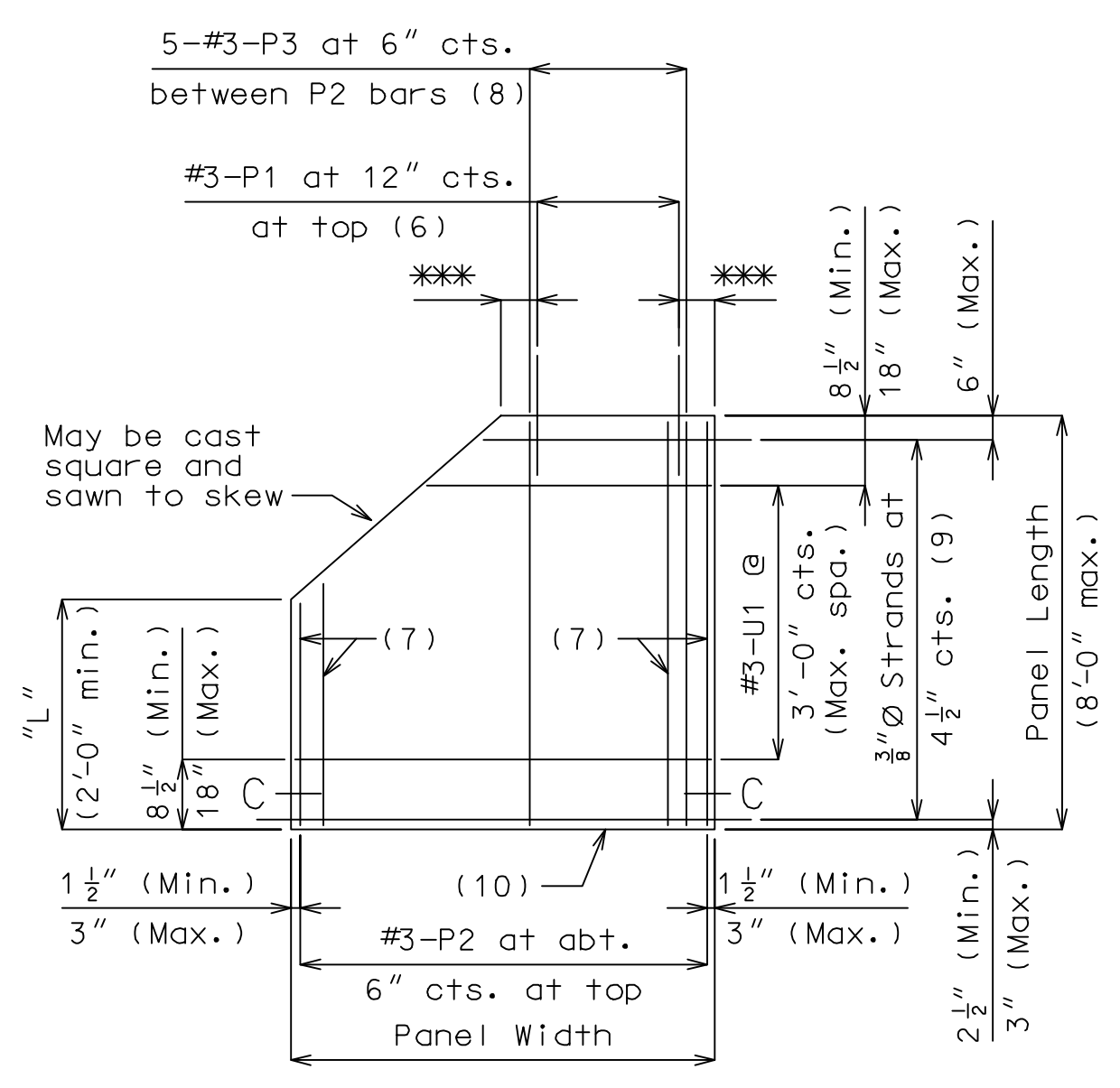


SECTION A-A
Reference Notes:

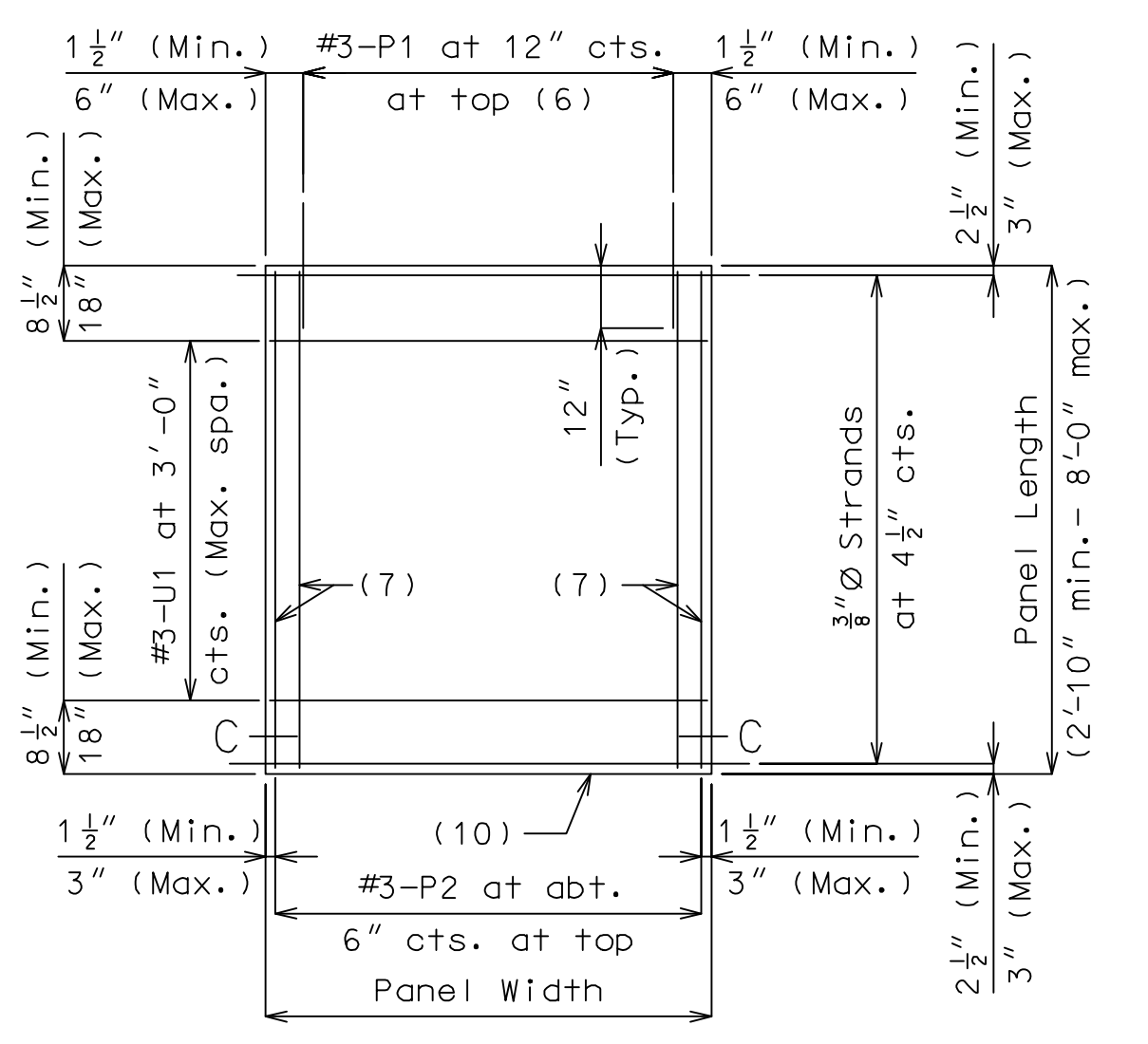
- Plan of Panels Placement:
- S-bars shown are bottom steel in slab between panels and used with squared and truncated end panels only.
 - Extend S-bars 18 inches beyond the front face of end bents and int. bents for squared and truncated end panels only.
 - Extend S-bars 9 inches beyond edge of girder (Typ.).
 - End panels shall be dimensioned 1/2" min. to 1 1/2" max. from the inside face of diaphragm.
 - For truncated end panels, use a min. of #5-S bars at 6" crossings in openings, or min. 4x4-W7xW7.
- Plans of Panels:
- For end panels only, P1 bars shall be 2'-0" in length and embedded 12". P1 bars will not be required for panels at squared integral end bents.
 - #3-P2 bars near edge of panel at bottom (under strands).
 - Use #3-P3 bars if panel is skewed 45° or greater.
 - Any strand 2'-0" or shorter shall have a #4 reinforcing bar on each side of it, centered between strands. Strands 2'-0" or shorter may then be deboned at the fabricator's option.
 - Optional 1/2" x 45° Chamfer one or both sides at bottom.
 - P1 bars not required for integral end bents.



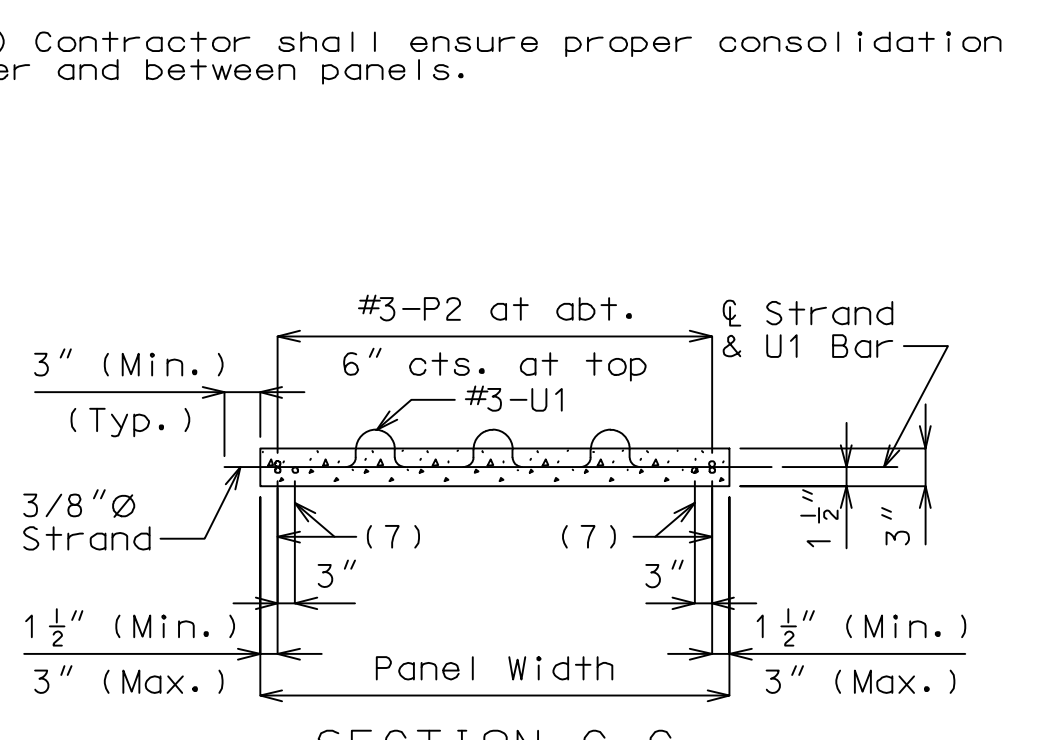
PLAN OF OPTIONAL SKEWED END PANEL



PLAN OF OPTIONAL TRUNCATED END PANEL
*** 3" (Min.), 6" (Max.)



PLAN OF SQUARED PANEL



SECTION C-C

DETAILS OF PRESTRESSED PANELS

General Notes:
Prestressed Panels:
Concrete for prestressed panels shall be Class A-1 with $f'c = 6,000$ psi, $f'ci = 4,000$ psi.
The top surface of all panels shall receive a scored finish with a depth of scoring of 1/8" perpendicular to the prestressing strands in the panels.
Prestressing tendons shall be high-tensile strength uncoated seven-wire low-relaxation strands for prestressed concrete in accordance with AASHTO M 203 Grade 270, with nominal diameter of strand = 3/8" and nominal area = 0.085 sq. in. and minimum ultimate strength = 22.95 kips (270 ksi). Larger strands may be used with the same spacing and initial tension.
Initial prestressing force = 17.2 kips/strand.

The method and sequence of releasing the strands shall be shown on the shop drawings.
Suitable anchorage devices for lifting panels may be cast in panels, provided the devices are shown on the shop drawings and approved by the engineer. Panel lengths shall be determined by the contractor and shown on the shop drawings.
When squared end panels are used at skewed bents, the skewed portion shall be cast full depth. No separate payment will be made for additional concrete and reinforcing required.
Support from diaphragm forms is required under the optional skewed end until cast-in-place concrete has reached 3,000 psi compressive strength.

Prestressed panels shall be brought to saturated surface-dry (SSD) condition just prior to the deck pour. There shall be no free standing water on the panels or in the area to be cast.
The prestressed panel quantities are not included in the table of estimated quantities for slab.

Reinforcing Steel:
All dimensions are out to out.
Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.
Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.
If U1 bars interfere with placement of slab steel, U1 loops may be bent over, as necessary, to clear slab steel.

Deformed welded wire reinforcement (WWR) providing a minimum area of reinforcing perpendicular to strands of 0.22 sq in./ft, with spacing parallel to strands sufficient to ensure proper handling, may be used in lieu of the #3-P2 bars shown. Wire diameter shall not be larger than 0.375 inch. The above alternative reinforcement criteria may be used in lieu of the #3-P3 bars, when required, and placed over a width not less than 2 feet.
The following reinforcing steel shall be tied securely to the strands with the following maximum spacing in each direction:
#3-P2 bars at 16 inches.
WWR at 24 inches.

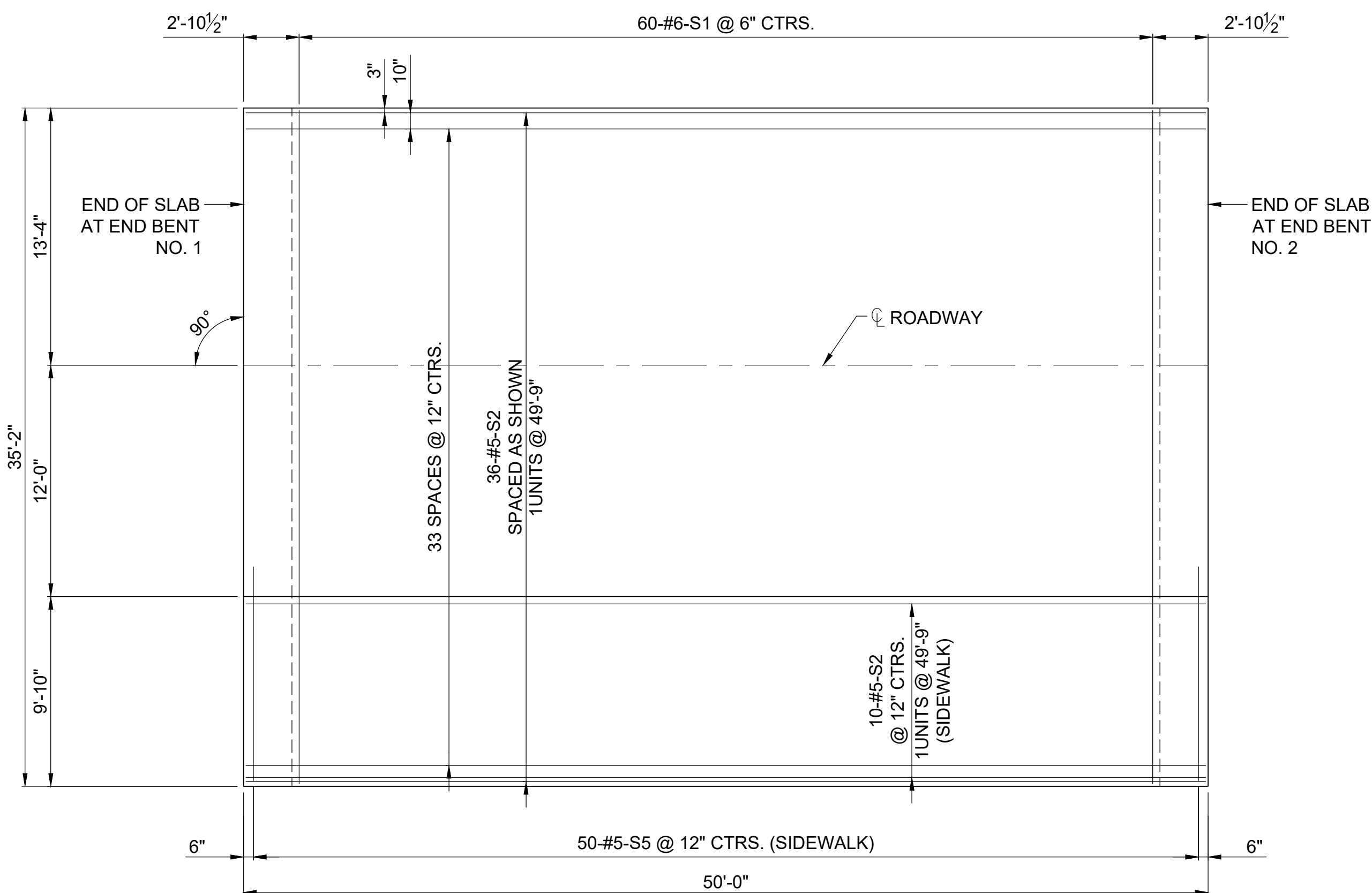
The #3-U1 bars shall be tied securely to #3-P2 bars, to WWR or to strands (when placed between P1 bars) at about 3-foot centers.
Minimum reinforcement steel length shall be 2'-0".
All reinforcement other than prestressing strands shall be epoxy coated.
Precast panels may be in contact with stirrup reinforcing in diaphragms.
S-bars are not listed in the bill of reinforcing.
Cost of S-bars will be considered completely covered by the contract unit price for the Slab.

Joint Filler:
Joint filler shall be preformed fiber expansion joint material in accordance with Sec 1057 or expanded or extruded polystyrene bedding material in accordance with Sec 1073.

The thickness of the joint filler shall be adjusted to achieve the slab haunching dimension found on Sheet No. 20. These adjustments shall be within the limits noted in the table of Joint Filler Dimensions.
Thicker material shall be used on one or both sides of the girder to reduce cast-in-place concrete thickness to within tolerances.

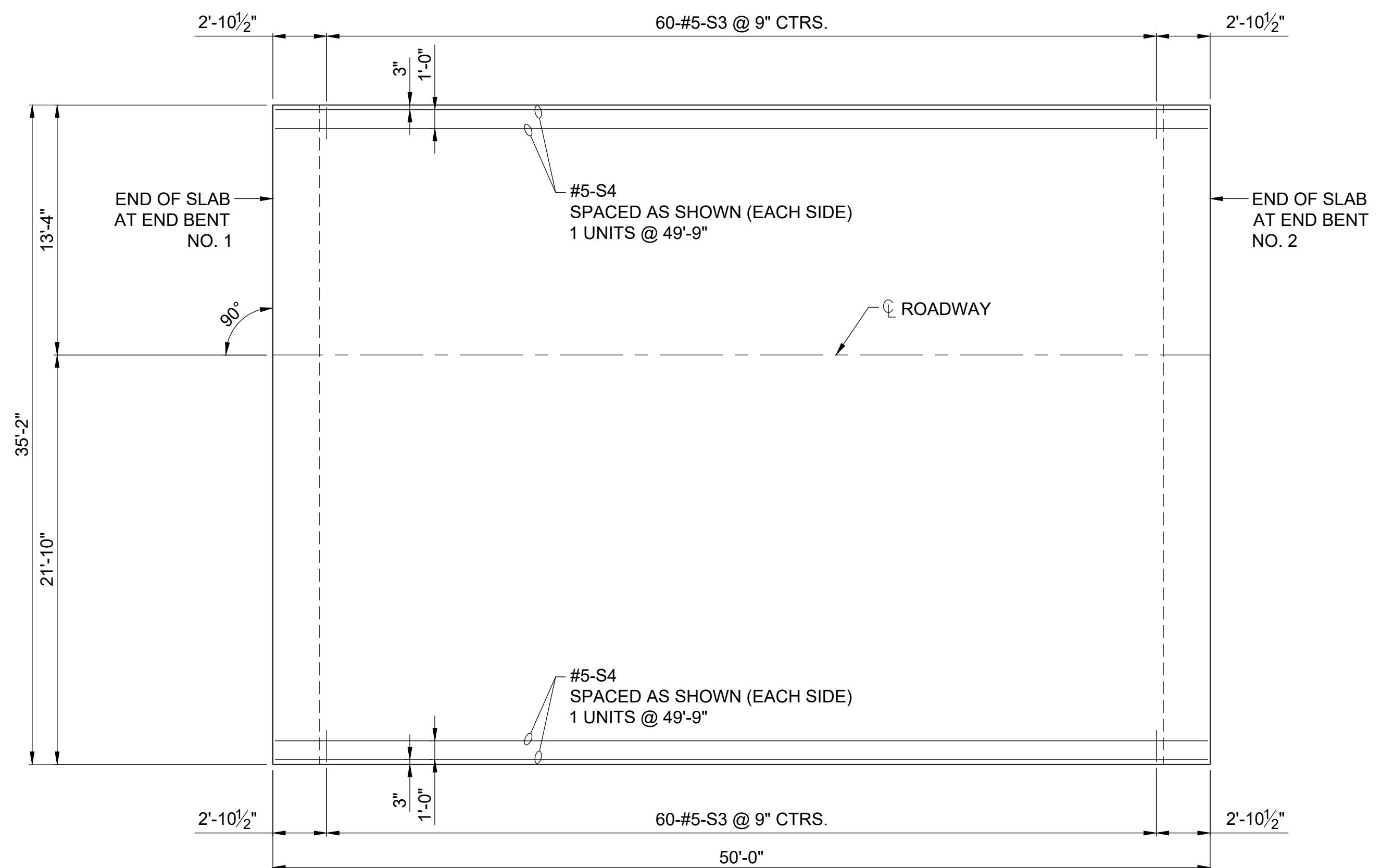
The same thickness of material shall be used under any one edge of any panel except at splices, and the maximum change in thickness between adjacent panels shall be 1/4 inch to correct for variations from Girder Camber Diagram. The polystyrene bedding material may be cut to match haunch height above top of flange.

Joint filler shall be glued to the girder. When thickness exceeds 1 1/2 inches, the joint filler shall be glued top and bottom. The glue used shall be the type recommended by the joint filler manufacturer.
Edges of panels shall be uniformly seated on the joint filler before slab reinforcement is placed.



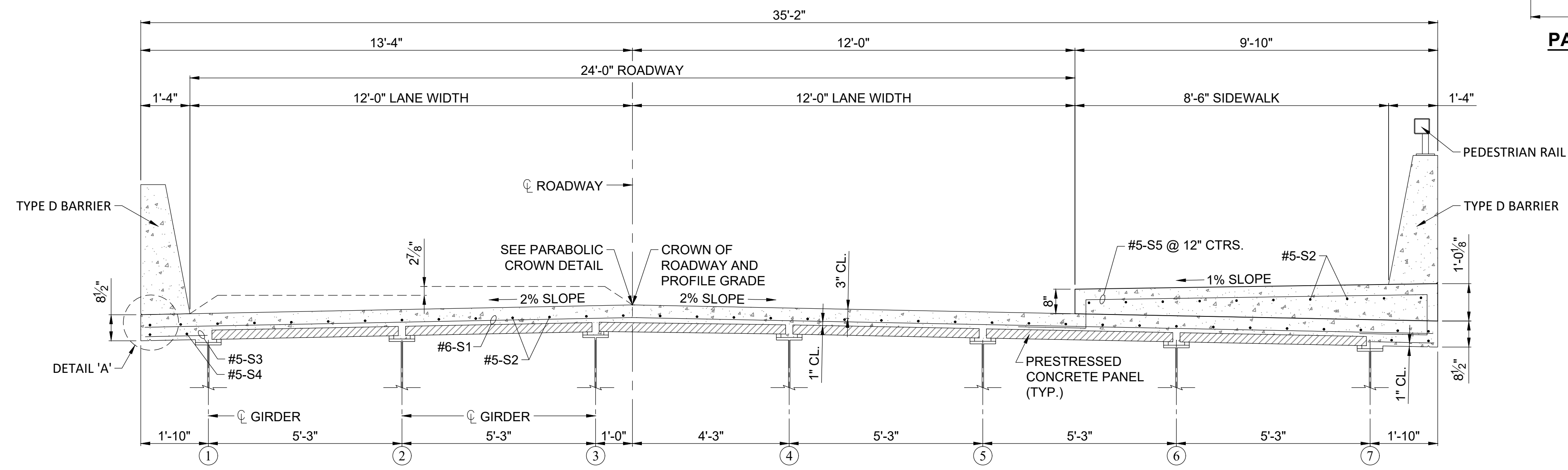
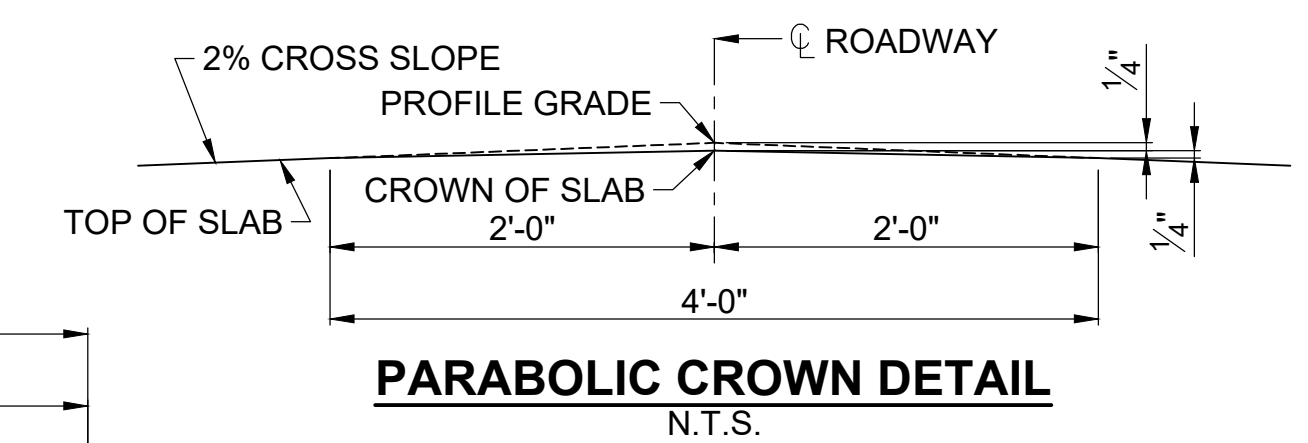
PLAN OF SLAB SHOWING TOP REINFORCEMENT

3/16" = 1'-0"



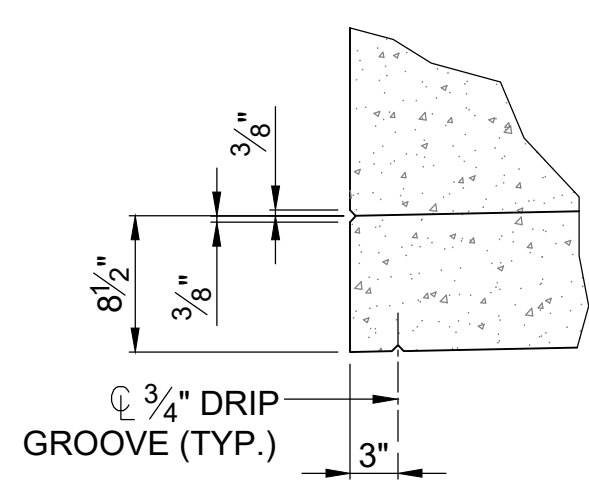
PLAN OF SLAB SHOWING BOTTOM REINFORCEMENT

3/16" = 1'-0"

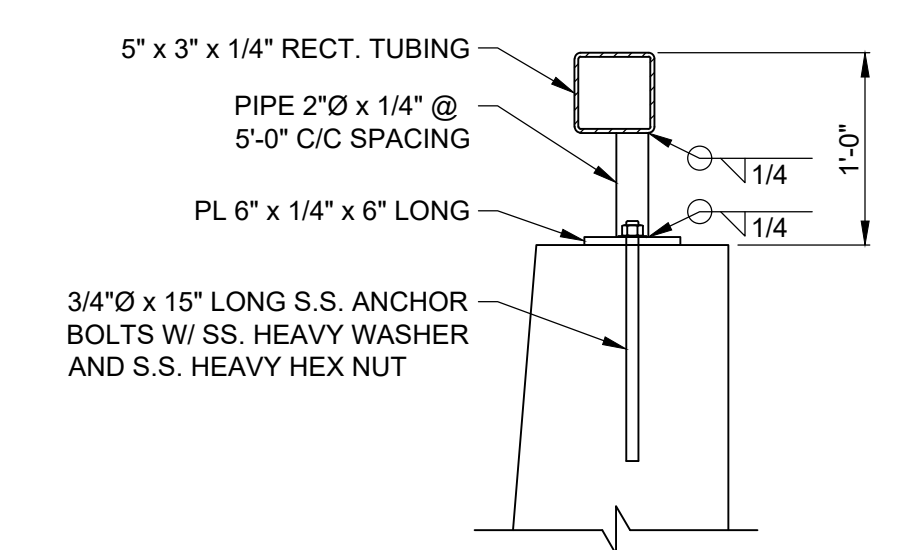


TYPICAL SECTION - BRIDGE

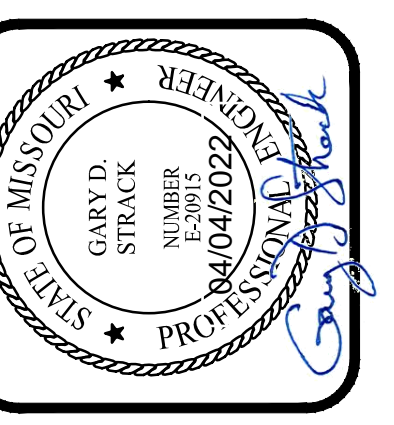
1/2" = 1'-0"



DETAIL 'A'
N.T.S.



PEDESTRIAN RAIL



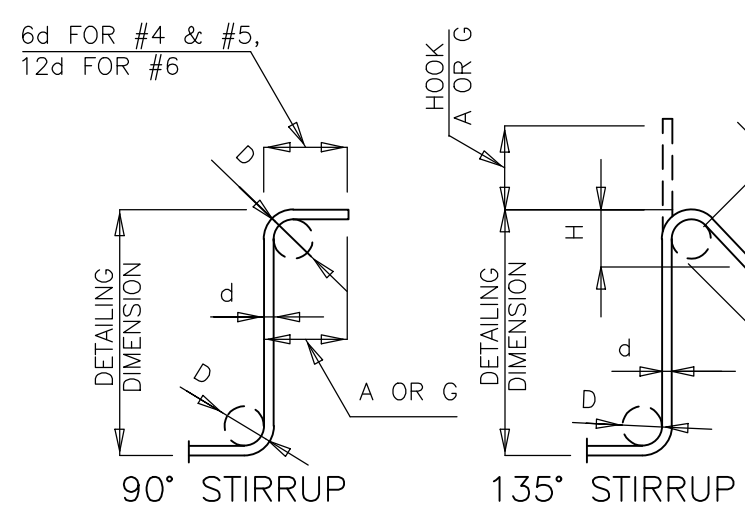
REVISIONS		DRAWING INFO.	
NO.	DESCRIPTION	DATE	BY

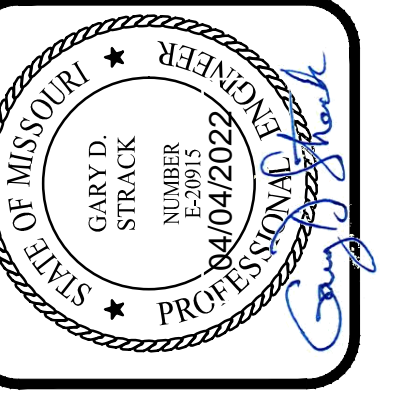
DRAWN BY:	CAW	FIELD BOOK:	20/CA0008
CHECK BY:	GDS	JOB NUMBER:	
DATE:	10/29/2020		

SLAB - PLAN & DETAILS
RIDGEMONT DR. BRIDGE OVER
COUNTY HOUSE BRANCH
SEC. 22 T48N, R13W
CITY OF COLUMBIA, MISSOURI

SHEET NUMBER
20
OF **28**

BILL OF REINFORCING STEEL																										
NO. REQ'D.	MARK NO.	LOCATION	EPOXY (E)	SHAPE NO.	STIRRUP (S)	SUBSTR. (X)	VARIES (V)	DIMENSIONS												NOMINAL LENGTH	ACTUAL LENGTH	WEIGHT				
								B		C		D		E		F		H					K			
								FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.				FT.	IN.	FT.	IN.
SUPERSTRUCTURE																										
END BENT 1																										
24	6F100	Wing	E 23	S				1	2.000	4	10.875	2	3.000	1	7.125	1	7.125	0	9.875	0	9.875	8	4	8	3	297
3	6F101	Diaphragm	E 19	S				4	9.875	2	2.000											7	0	6	11	31
3	6F102	Diaphragm	E 19	S				8	2.500	2	2.000											10	5	10	4	47
6	6F103	Diaphragm	E 15	S				2	9.000	3	8.125	2	3.000	1	7.125	1	7.000	1	11.250	1	11.375	8	8	8	7	77
4	6H100	Beam	E 20					41	8.000												41	8	41	8	250	
8	7H101	Beam	E 20					41	8.000												41	8	41	8	681	
6	6H102	Diaphragm	E 20					34	10.125												34	10	34	10	314	
24	8H103	Diaphragm	E 20					9	5.500												9	6	9	6	609	
52	6H104	Diaphragm	E 20					8	7.500												8	8	8	8	677	
4	7H105	Diaphragm	E 20					13	6.000												13	6	13	6	110	
4	8H106	Wing	E 20					34	10.125												34	10	34	10	372	
21	5U100	Beam	E 31	S				5	1.625	2	2.000	5	1.625								13	4	13	2	288	
16	5U101	Beam	E 13	S				2	2.000	3	1.000	2	2.000	3	1.000						11	5	11	1	185	
6	5U102	Beam	E 10	S				3	1.000	2	2.000										8	4	8	2	51	
43	6U103	Diaphragm	E 19	S				2	3.625	4	6.000										6	10	6	8	431	
26	6U104	Diaphragm	E 31	S				2	2.125	2	2.000	2	2.125								7	5	7	3	283	
3	5U105	Diaphragm	E 10	S				1	10.000	1	0.000										4	8	4	6	14	
9	5U106	Beam	E 13	S				2	2.000	3	3.625	2	2.000	3	3.625						11	10	11	6	108	
10	5V100	Beam	E 20					5	2.125												5	2	5	2	54	
14	6V101	Diaphragm	E 20					2	2.500												2	3	2	3	47	
2	6V102	Wing	E 20					6	10.000												6	10	6	10	21	
14	6V103	Wing	E 20		V	2		6	10.000												6	10	6	10	144	
		INCREMENT =						6	10.625												6	11				
2	6V104	Wing	E 20					6	0.125												6	0	6	0	18	
14	6V105	Wing	E 20		V	2		6	0.250												6	0	6	0	127	
		INCREMENT =						6	0.750												6	1				
2	6V106	Wing	E 20					5	10.375												5	10	5	10	18	
14	6V107	Wing	E 20		V	2		5	10.500												5	11			124	
		INCREMENT =						5	11.125												5	11				
12	6V108	Wing	E 20					5	10.875												5	11	5	11	107	
END BENT 2																										
24	6F100	Wing	E 23	S				1	2.000	4	10.875	2	3.000	1	7.125	1	7.125	0	9.875	0	9.875	8	4	8	3	297
3	6F101	Diaphragm	E 19	S				4	9.875	2	2.000										7	0	6	11	31	
3	6F102	Diaphragm	E 19	S				8	2.500	2	2.000										10	5	10	4	47	
6	6F103	Diaphragm	E 15	S				2	9.000	3	8.125	2	3.000	1	7.125	1	7.000	1	11.250	1	11.375	8	8	8	7	77
4	6H100	Beam	E 20					41	8.000												41	8	41	8	250	
8	7H101	Beam	E 20					41	8.000												41	8	41	8	681	
6	6H102	Diaphragm	E 20					34	10.125												34	10	34	10	314	
24	8H103	Diaphragm	E 20					9	5.500												9	6	9	6	609	
52	6H104	Diaphragm	E 20					8	7.500												8	8	8	8	677	
4	7H105	Diaphragm	E 20					13	6.000												13	6	13	6	110	
4	8H106	Wing	E 20					34	10.125												34	10	34	10	372	
21	5U100	Beam	E 31	S				5	1.625	2	2.000	5	1.625								13	4	13	2	288	
16	5U101	Beam	E 13	S				2	2.000	3	1.000	2	2.000	3	1.000							11	5	11	1	185
6	5U102	Beam	E 10	S				3	1.000	2	2.000										8	4	8	2	51	
43	6U103	Diaphragm	E 19	S				2	3.625	4	6.000										6	10	6	8	431	
26	6U104	Diaphragm	E 31	S				2	2.125	2	2.000	2	2.125								7	5	7	3	283	
3	5U105	Diaphragm	E 10	S				1	10.000	1	0.000										4	8	4	6	14	
9	5U106	Beam	E 13	S				2	2.000	3	3.625	2	2.000	3	3.625						11	10	11	6	108	
10	5V100	Beam	E 20					5	2.125												5	2	5	2	54	
14	6V101	Diaphragm	E 20					2	2.500												2	3	2	3	47	
2	6V102	Wing	E 20					6	10.000												6	10	6	10	21	
14	6V103	Wing	E 20		V	2		6	10.000												6	10	6	10	144	
		INCREMENT =						6	10.625												6	11				
2	6V104	Wing	E 20					6	0.125												6	0	6	0	18	
14	6V105	Wing	E 20		V	2		6	0.250												6	0	6	0	127	
		INCREMENT =						6	0.750												6	1				
2	6V106	Wing	E 20					5	10.375												5	10	5	10	18	
14	6V107	Wing	E 20		V	2		5	10.500												5	11			124	
		INCREMENT =						5	11.125												5	11				
12	6V108	Wing	E 20					5	10.875												5	11	5	11	107	
SLAB																										
60	5S1	Slab	E 20					34	11.000												34	11	34	11	3,147	
46	5S2	Slab	E 20					49	9.000												49	9	49	9	2,387	
120	5S3	Slab	E 20					1	8.125												1	8	1	8	209	
4	5S4	Slab	E 20					49	9.000												49	9	49	9	208	
50	5S5	Slab	E 11					1	10.000	1	0.625	9	3.000		8.750						14	8	14	3	743	

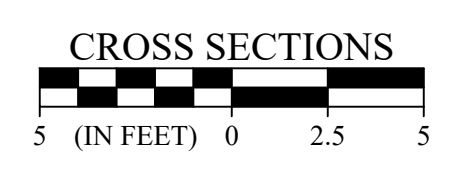
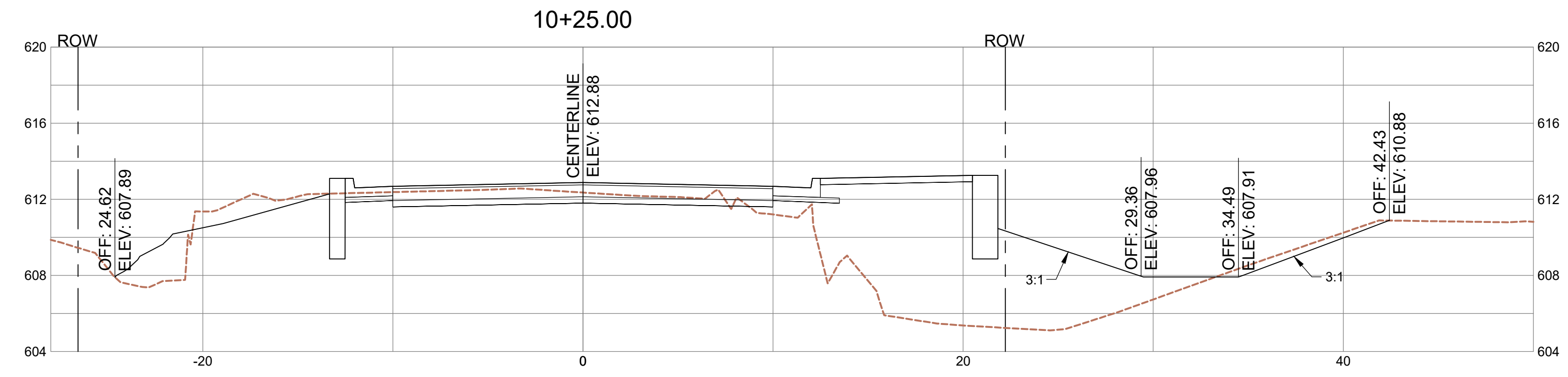
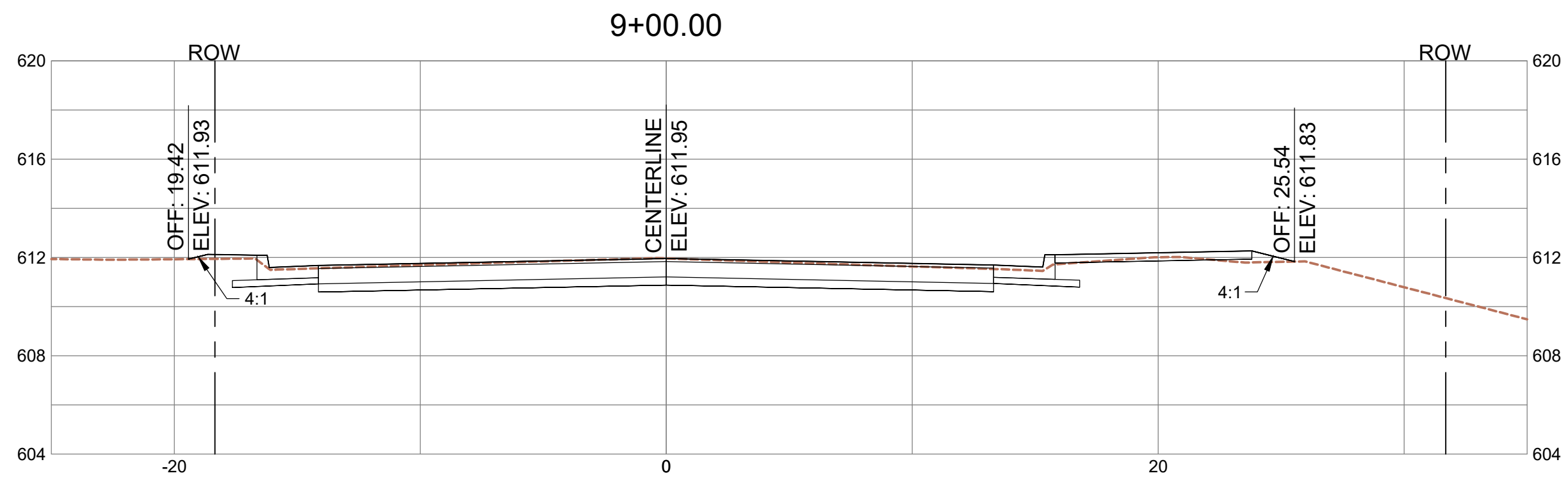
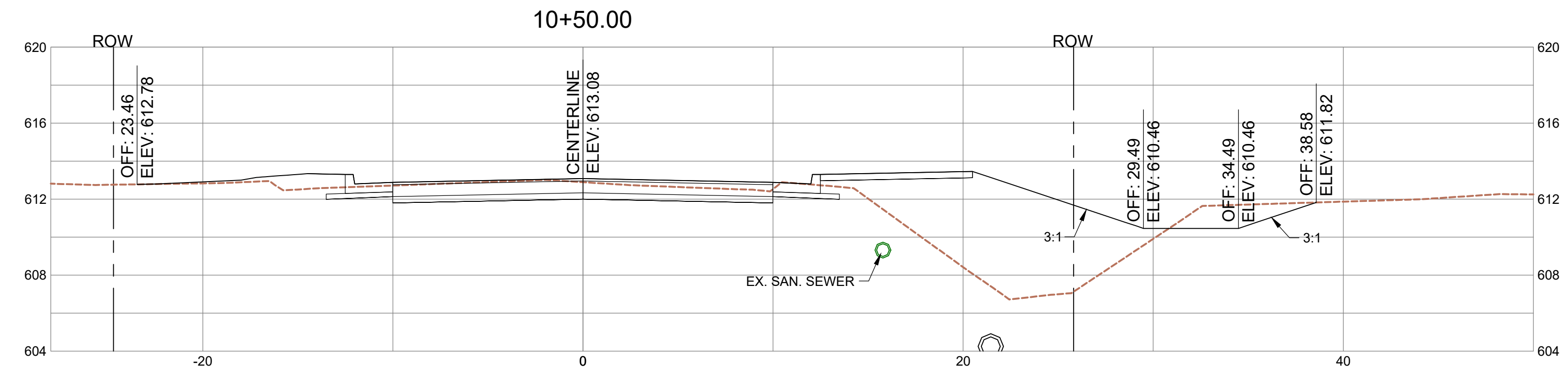
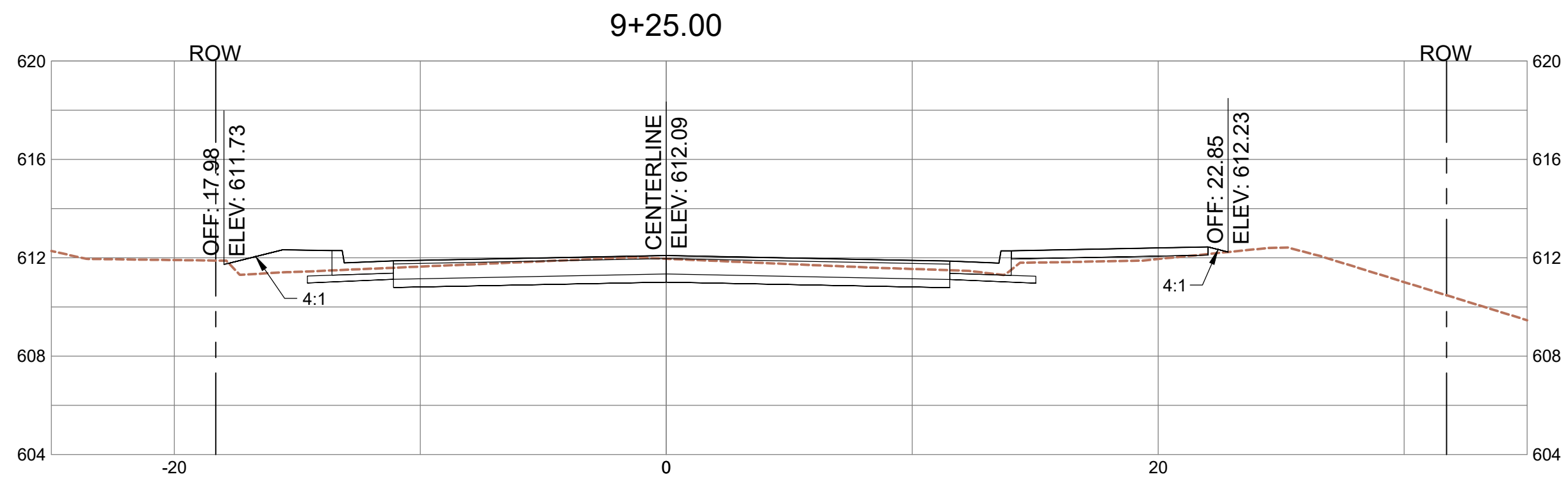
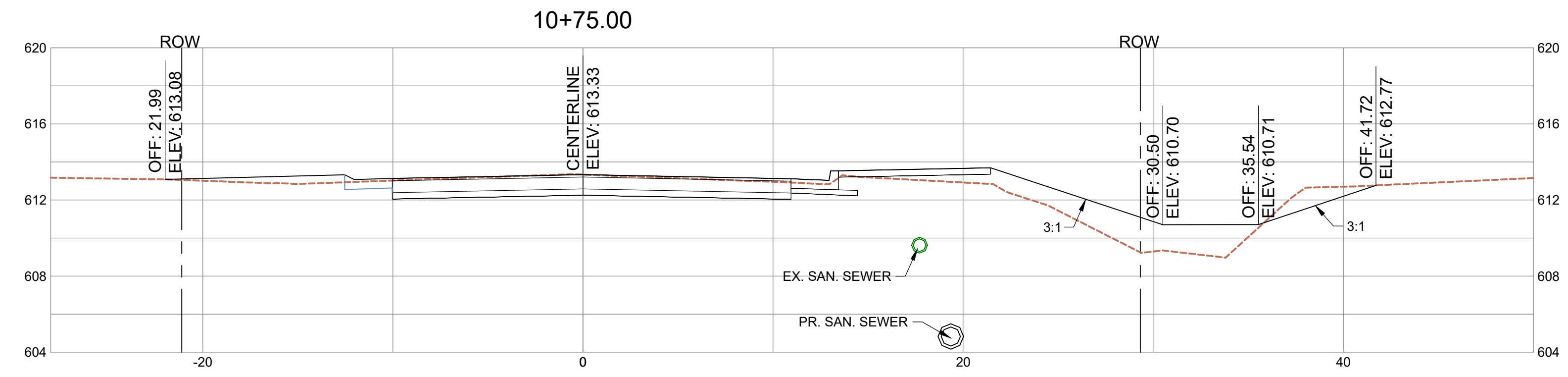
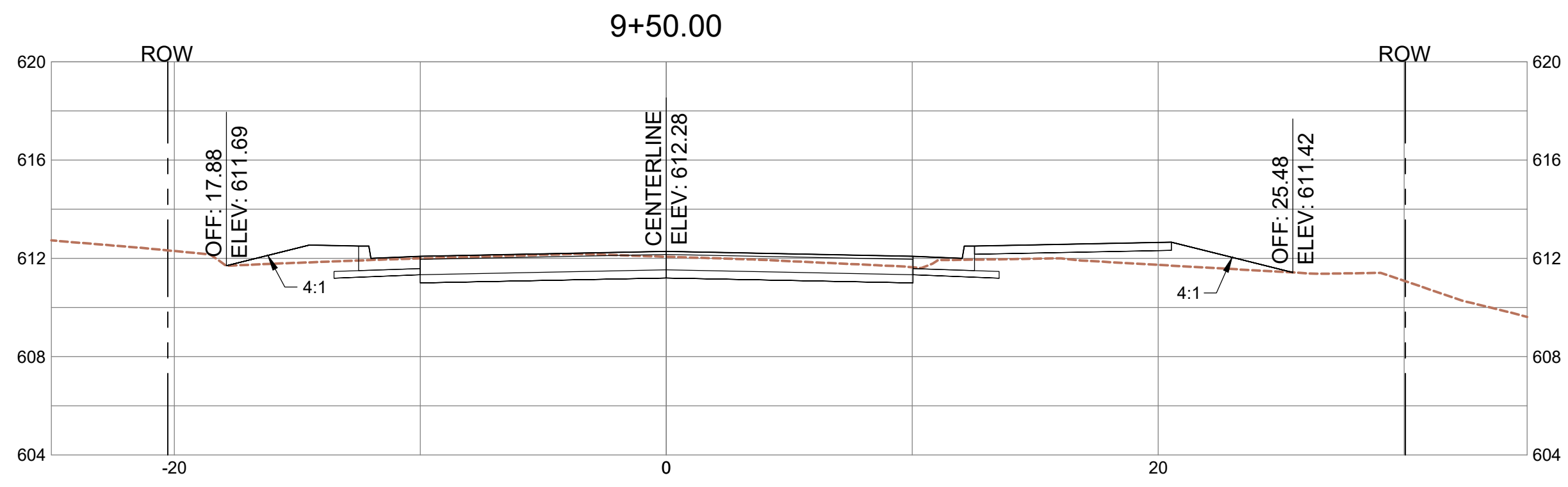
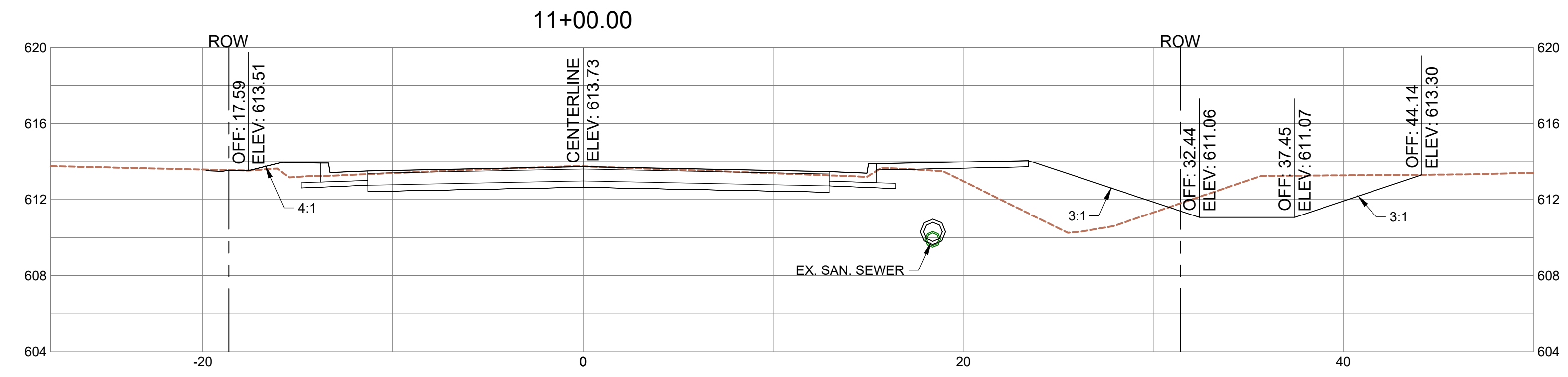
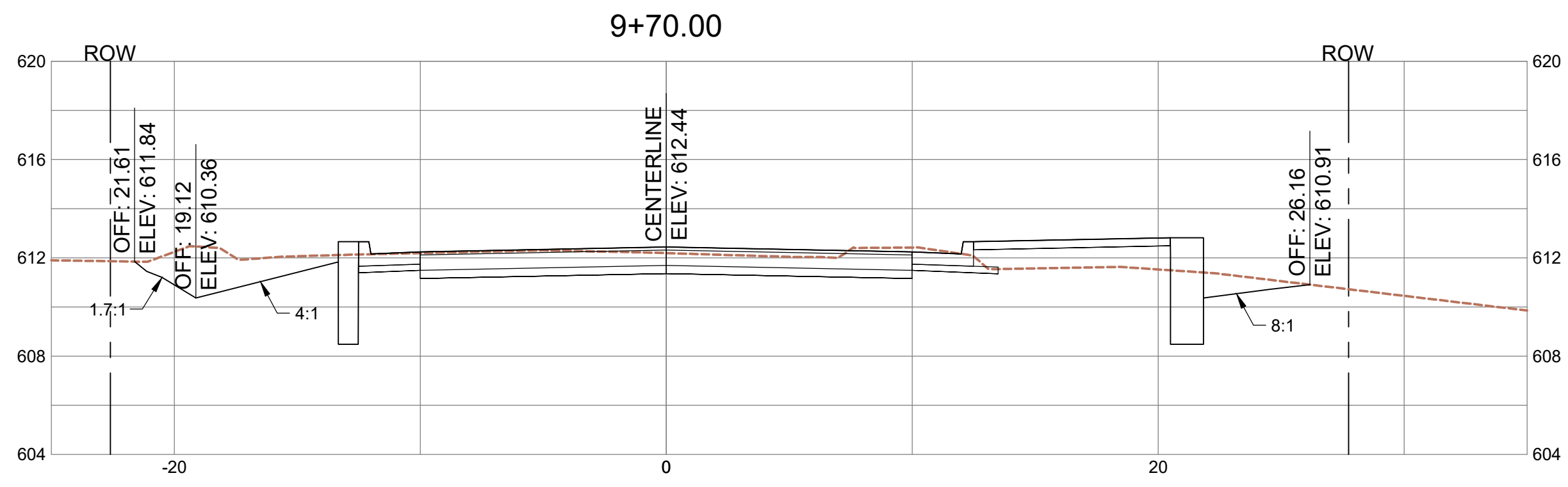




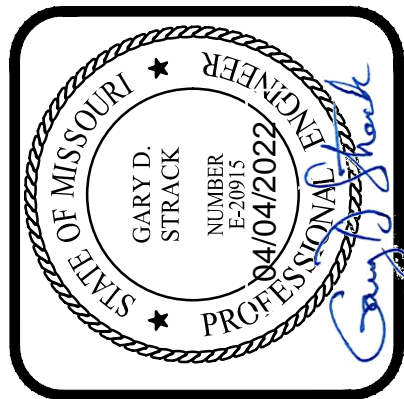
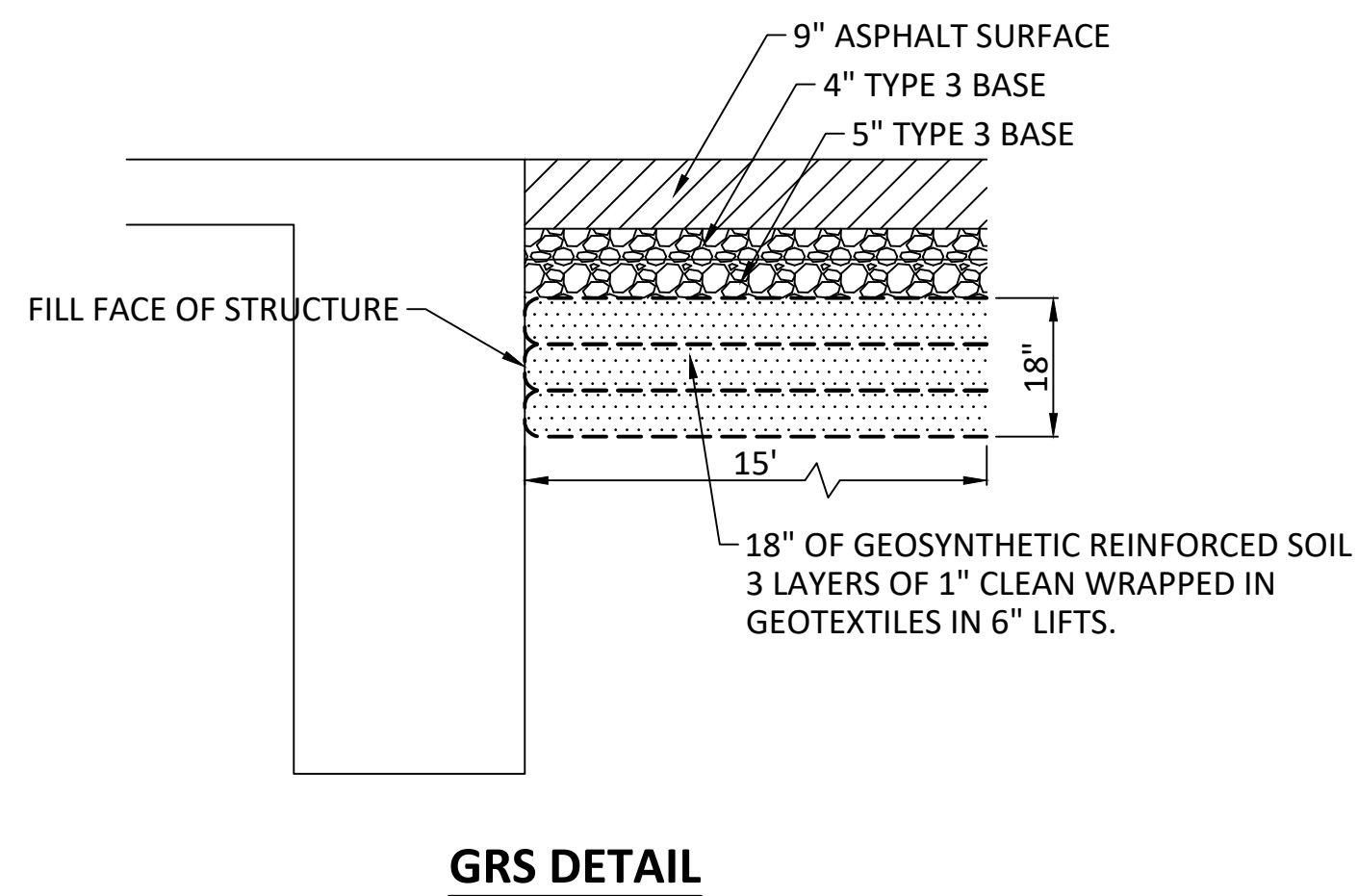
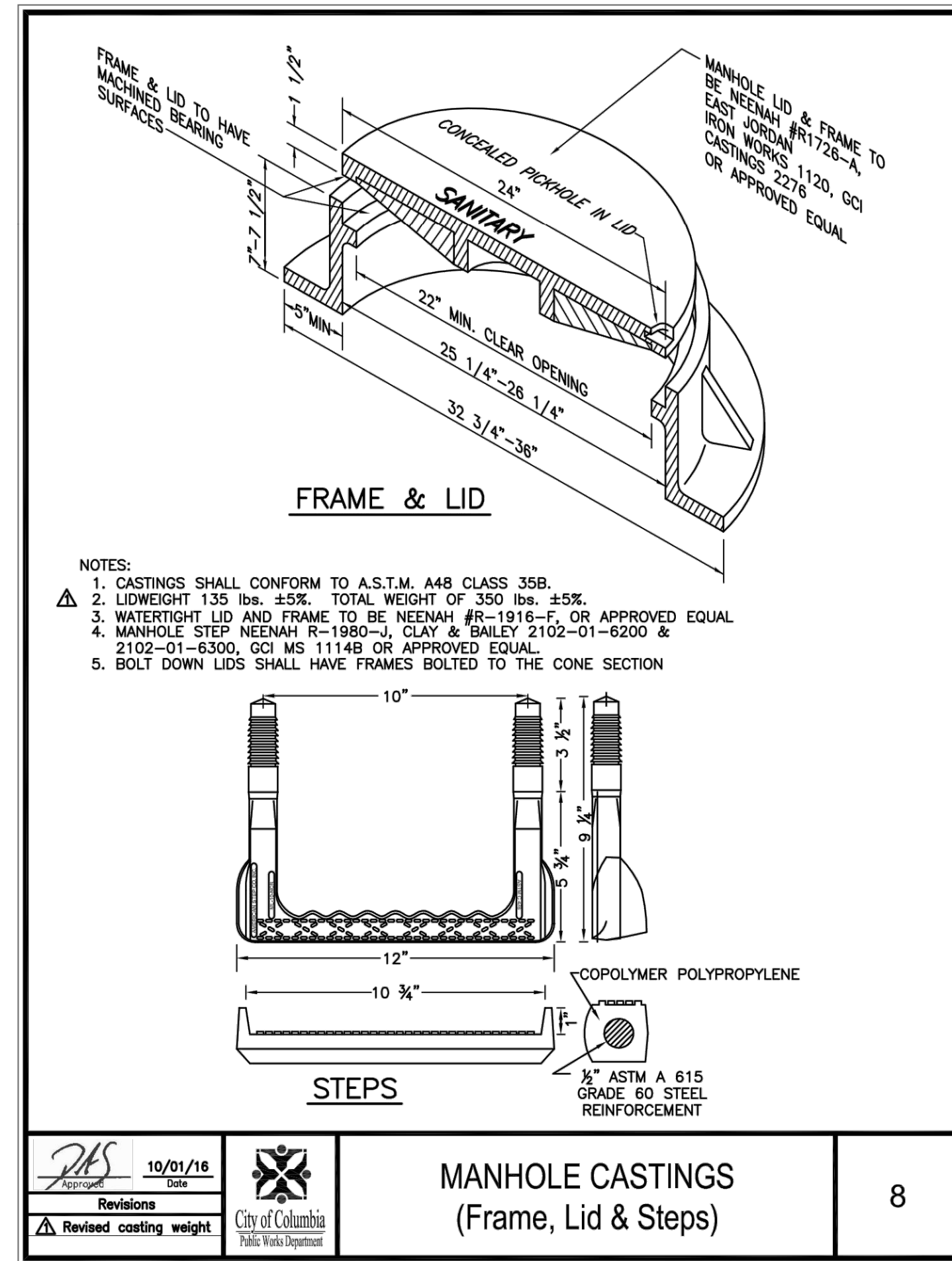
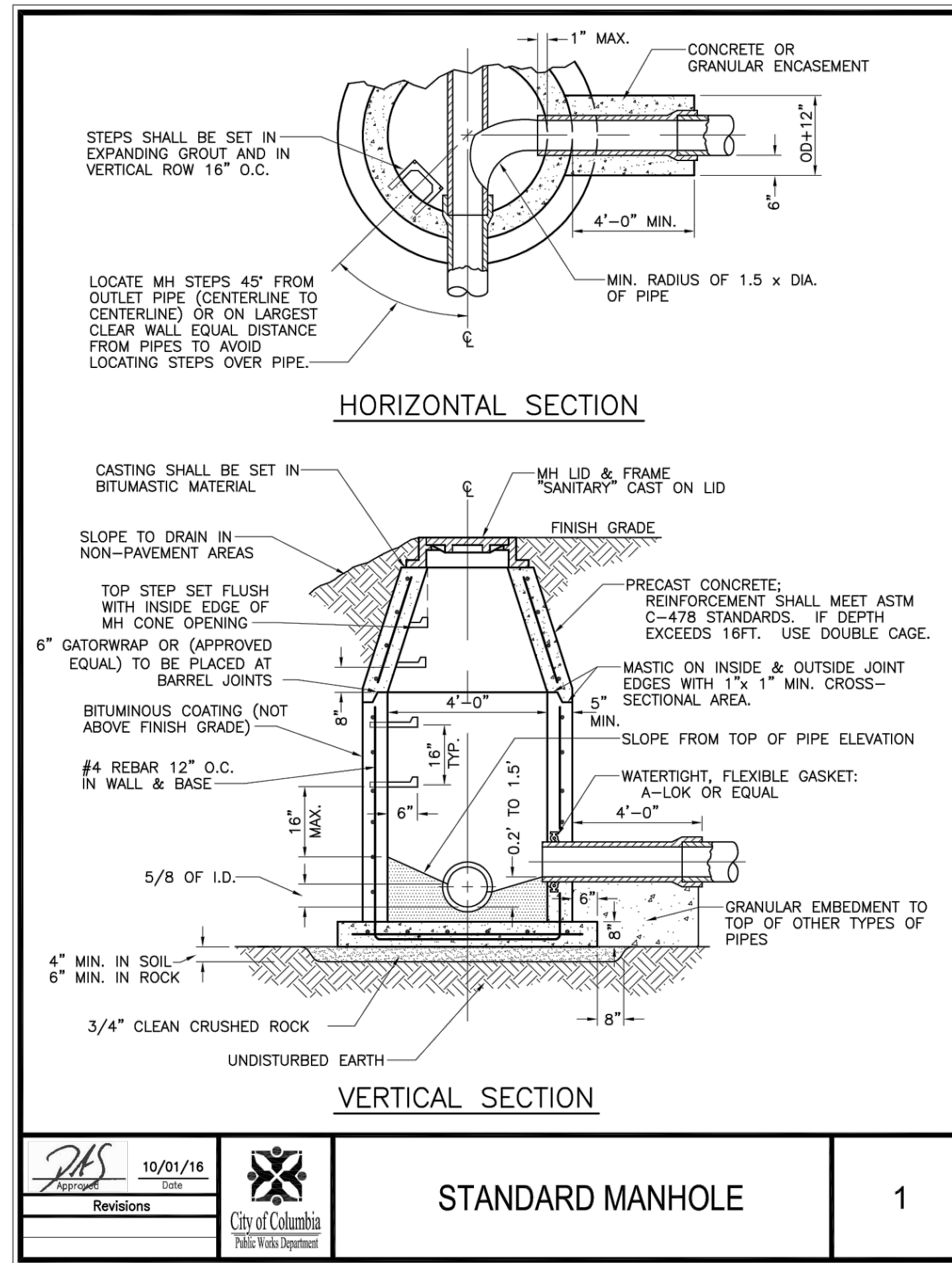
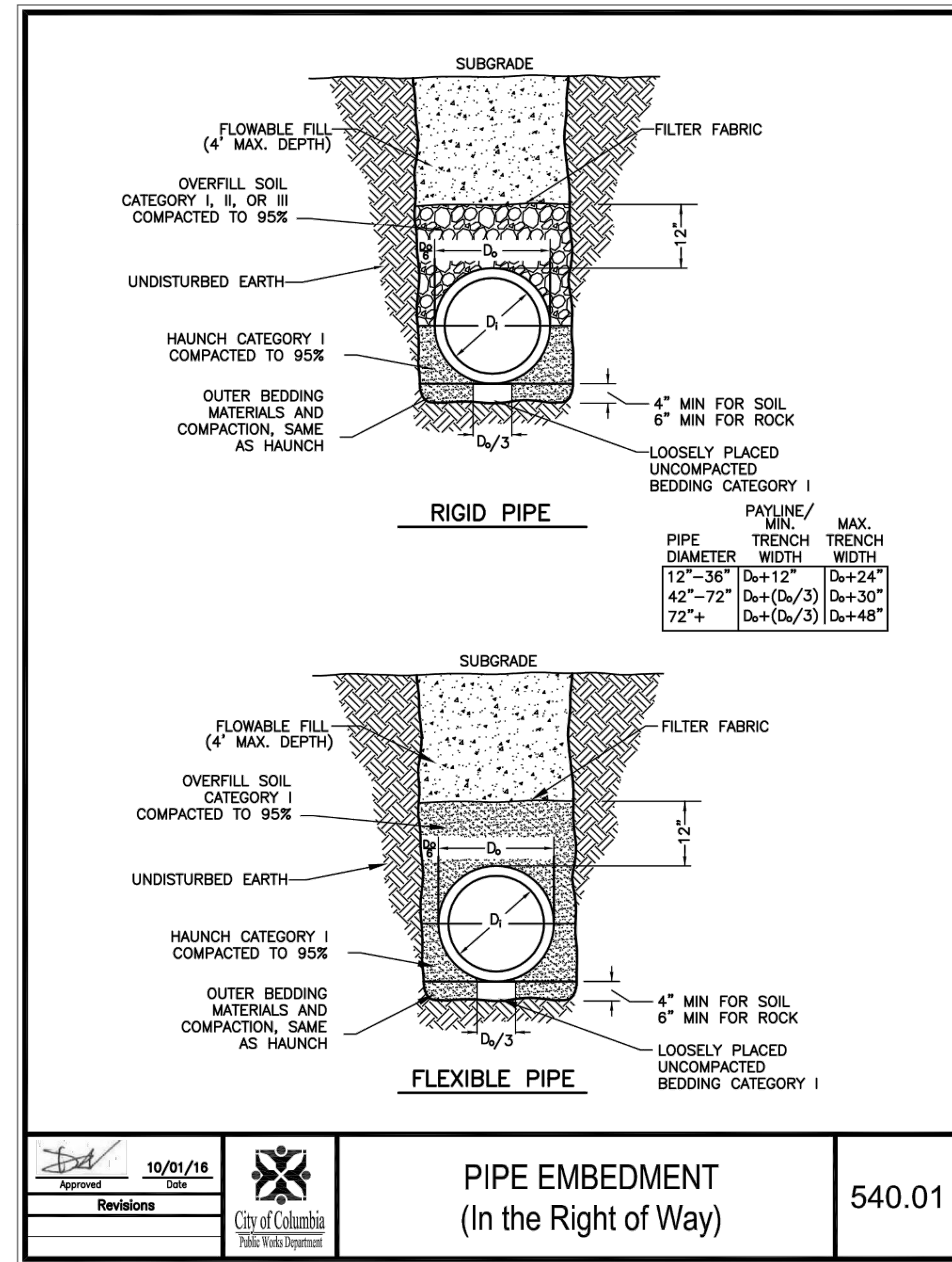
REVISIONS		DRAWING INFO.			
NO.	DESCRIPTION	BY	DATE	FIELD BY:	NO./DATE
				DRAWN BY:	ND/CAW
				CHECK BY:	GDS
				DATE:	10/21/2021
				FIELD BOOK:	
				JOB NUMBER:	2012CA0008

CROSS SECTIONS
 RIDGEMONT DR. BRIDGE OVER
 COUNTY HOUSE BRANCH
 SEC. 22 T48N, R13W
 CITY OF COLUMBIA, MISSOURI

SHEET NUMBER
25
 OF **28**



©Shawndr dmvw03 ND/CAW/MS/CAD/0008 Ridgmont Bridge/CAD/Plan/1/Cross Sections



DRAWING INFO.		FIELD BY:		DATE	
NO.	DESCRIPTION	NO./DRAWN	DATE	NO./CHECKED	DATE
		ND/DCAW	10/21/2021	GDS	10/21/2021
				FIELD BOOK:	
				JOB NUMBER:	20NC40008

DETAILS
 RIDGEMONT DR. BRIDGE OVER
 COUTNY HOUSE BRANCH
 SEC. 22 T48N, R13W
 CITY OF COLUMBIA, MISSOURI

SHEET NUMBER
 28
 OF 28